

STATE DEPARTMENT OF HIGHWAYS

DIVISION OF HIGHWAYS—STATE OF COLORADO

PE, ROW AND UTILITIES
UNDER IXFU 470-1(58)
AND IXFU 470-1(12)

FEDERAL ROAD PROJECT NO.	DIVISION	PROJECT NO.	SHEET NO.
IXFU 470-1(55)	COLORADO	IXFU 470-1(55)	1

AS CONSTRUCTED			
NO REVISIONS	REVISED	7-15-88	VOICD

REVISIONS			

TABULATION OF LENGTH AND DESIGN DATA

STATION TO STATION S.H. 470	ROADWAY	LINEAR FEET MAJOR STRUCTURE
357+60 BEGIN IXFU 470-1(55) = 357+60 ON IXFU 470-1(51) MILE POST 6.15	340.0	
361+00 END IXFU 470-1(55) = 361+00 ON IXFU 470-1(51) MILE POST 6.21		
SUBTOTAL S.H. 470	340.00	
QUINCY 111+00 BEGIN QUINCY CONSTRUCTION 113+84.50 BEGIN STR. NO. F-16-NE	284.60	294.64
116+79.24 END STR. NO. F-16-NE 118+00 END QUINCY CONSTRUCTION	120.75	
SUBTOTAL QUINCY	405.36	294.64
PROJECT NET LENGTH	745.36	294.64

SUMMARY S.H. 470		
ROADWAY IXFU 470-1(55) (NET LENGTH)	745.36	0.141
MAJOR STRUCTURES IXFU 470-1(55)	294.64	0.056
PROJECT GROSS LENGTH	1040.00	0.197

DESIGN DATA		
MAXIMUM DEGREE OF CURVE	SH-470 0.30'	QUINCY 16°22'13"
MAXIMUM GRADE	-0.50%	+3.82%
MINIMUM SSD HORIZONTAL	600'	450'
MINIMUM SSD VERTICAL	910'	440'
MAXIMUM DESIGN SPEED	70 MPH	30 MPH
EIS DESIGN TRAFFIC VOLUME (YEAR 2,000)	ADT = 52,800 DHV = 5,280	28,640 3,150

PLAN AND PROFILE OF PROPOSED AS CONSTRUCTED

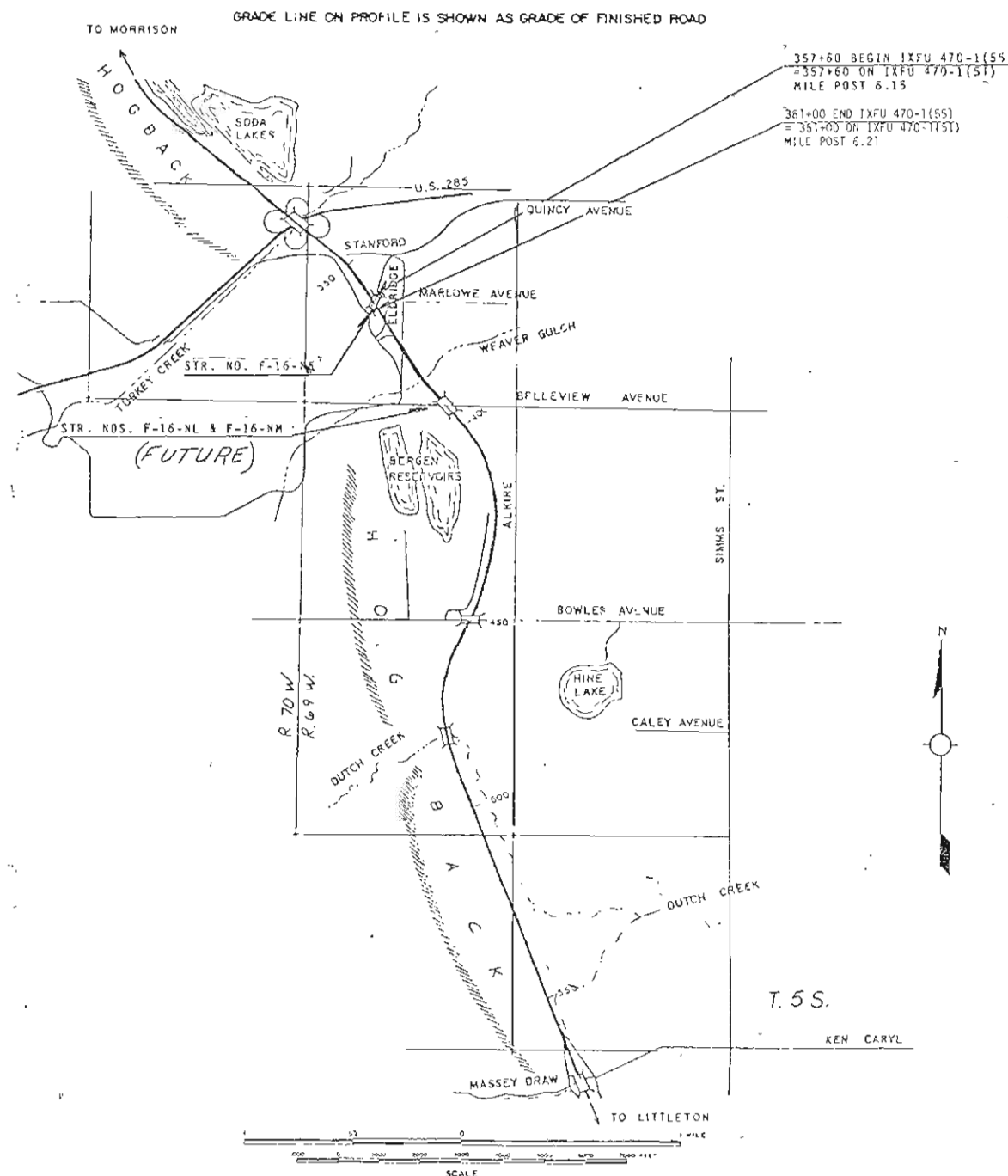
COMBINED FEDERAL AID PROJECT NOS. IXFU 470-1(55) & DE 0200(801)

COMBINED WITH FEDERAL AID PROJECT NO. IXFU 470-1(45) & DE 0200(802) FOR CONSTRUCTION

STATE HIGHWAY NO. 470

JEFFERSON COUNTY

SCALES OF ORIGINAL DRAWINGS
AS SHOWN ON PLAN AND PROFILE



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NEW AND REVISED STANDARDS
FIELD LABORATORY CLASS 2-M-520-2-MAY 25, 1988

DIVISION OF HIGHWAYS

APPROVED:

James E. Diehl
Asst. Chief Engineer
5-13-88
DATE

AS CONSTRUCTED INFORMATION

CONTRACTOR FLATIRON TERRAZZO CO.
ENGINEER K. L. ALLEN
(Project or Resubmit)
PROJECT STARTED 1-2-88
PROJECT COMPLETED 7-15-88
Cons. Fee 282.90
TITLE DATE

AS CONSTRUCTED		
NO. REVISIONS	7-15-81	REVISED
		VOID

FEDERAL ROAD REGION NO.	DIVISION	PROJECT NO.	SHEET NO.	TOTAL SHEETS
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THE STANDARD PLAN SHEETS INDICATED HEREON BY A MARKED BOX ARE TO BE USED TO CONSTRUCT THIS PROJECT.

DEPARTMENT OF HIGHWAYS
STATE OF COLORADO
DIVISION OF HIGHWAYS

STANDARD PLANS LIST

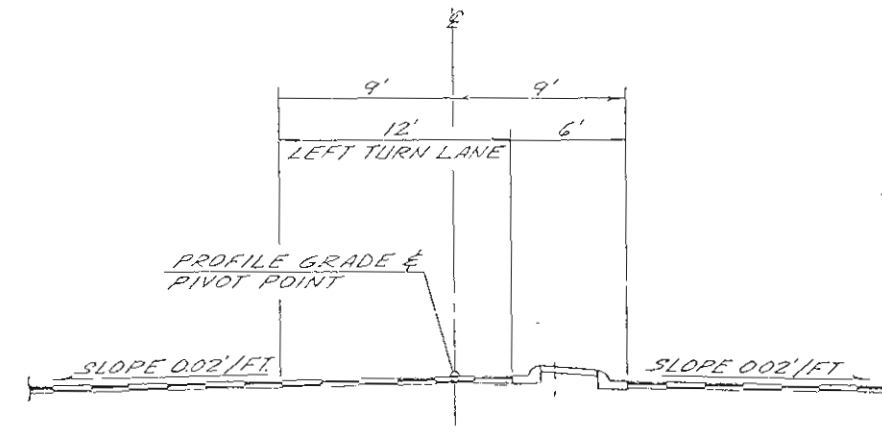
M & S STANDARDS JANUARY, 1982

TYPICAL SECTIONS

AS CONSTRUCTED		
NO REVISIONS	7-15-97	REVISED
VOID		

FEDERAL ROAD DISTRICT NO.	DIVISION	PROJECT NO.	SHEET NO.	TOTAL SHEETS
001	COLORADO	IXFU 470-1 (35)	4	49

REVISIONS		



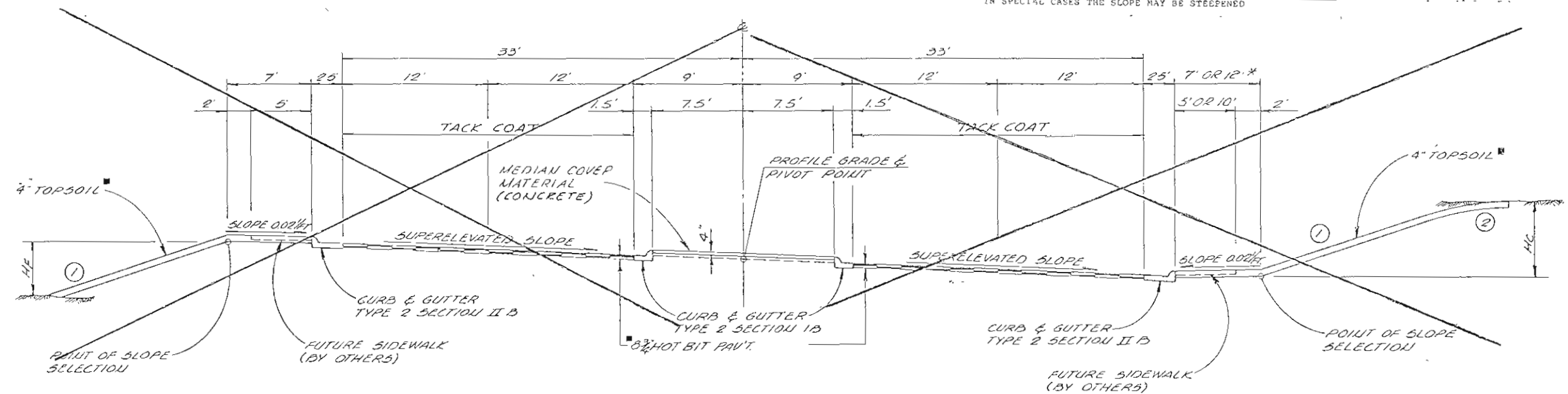
LEFT TURN LANE

FILL SLOPE	CUT SLOPE
HF	HC
10' OR LESS	10' OR LESS
10' TO 25'	10' TO 15'
OVER 25'	OVER 15'
SLOPE	SLOPE
6:1	6:1
4:1	4:1
3:1	3:1

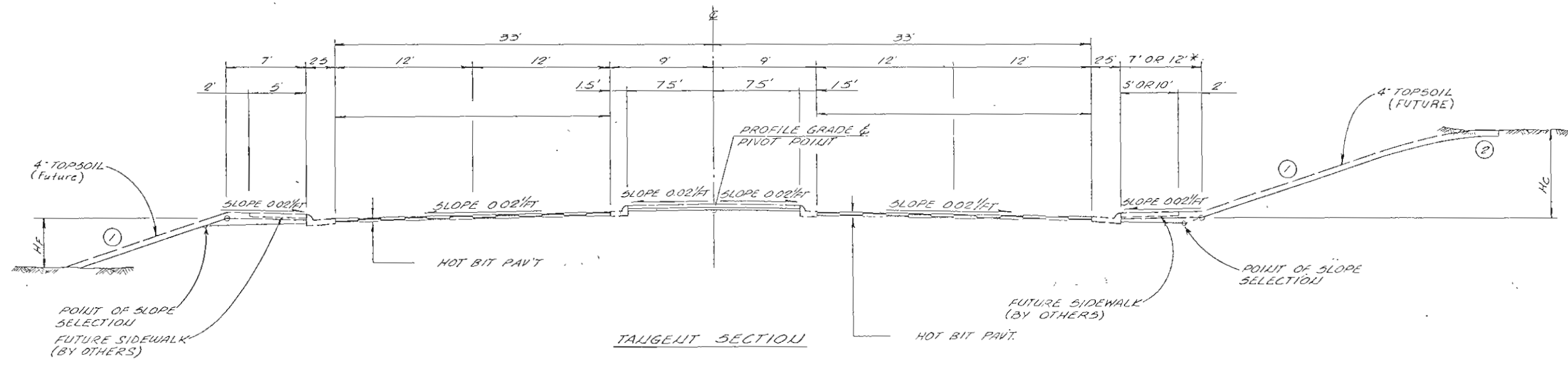
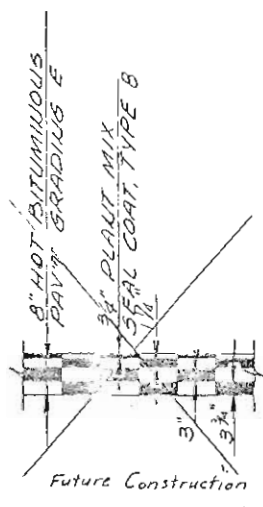
FILL SLOPE	CUT SLOPE
HF	HC
10' OR LESS	10' OR LESS
10' TO 25'	10' TO 15'
OVER 25'	OVER 15'
SLOPE	SLOPE
6:1	6:1
4:1	4:1
3:1	3:1

IN SPECIAL CASES THE SLOPE MAY BE STEEPENED

- NOTES:
- FOR CUT AND FILL SLOPES SEE TABLES THIS SHEET
 - SEE STANDARD M-203-1 FOR DETAILS OF CUT SLOPE TREATMENT, FLARING AND WIDENING
- THE DEPTH AND WIDTH OF THE SIDE SLOPE SHALL BE VARIED WHERE NECESSARY IN ORDER TO PROVIDE PROPER DRAINAGE
- BREAK POINTS ON SLOPES AND IN BOTTOMS OF DITCHES SHALL BE ROUNDED IN CONSTRUCTION FOR A PLEASING APPEARANCE



SUPERELEVATED SECTION



TANGENT SECTION

QUINCY AVENUE

AS CONSTRUCTED			FED. ROAD REGION	DIVISION	PROJ. NO.	SHEET NO.	SHEET TOTALS
NO REVISIONS	7-15-81	REVISED	VOID	VIII	COLO.	IXFU 470-1(55)	5 49

GENERAL NOTES

WATER SHALL BE USED AS A DUST PALLIATIVE WHERE REQUIRED. LOCATIONS SHALL BE AS ORDERED. IT IS ESTIMATED THAT 100 MGAL. WILL BE REQUIRED FOR THIS PROJECT AND WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE COST FOR EMBANKMENT MATERIAL (C.I.P.).

DEPTH OF MOISTURE DENSITY CONTROL FOR THIS PROJECT SHALL BE AS FOLLOWS:
FULL DEPTH OF ALL EMBANKMENTS
BASES OF CUT AND FILLS 0.50 FOOT

EXCAVATION REQUIRED FOR COMPACTION OF BASES OF CUTS AND FILLS WILL BE CONSIDERED AS SUBSIDIARY TO THAT OPERATION AND WILL NOT BE PAID FOR SEPARATELY.

TYPE OF COMPACTION FOR THIS PROJECT WILL BE AASHTO T-99. SUBEXCAVATION MATERIAL SHALL BE PLACED IN 8-INCH LIFTS AND COMPACTED TO NO LESS THAN 92% STANDARD DENSITY (AASHTO T-99) AND NO LESS THAN OPTIMUM MOISTURE.

6.0 FEET OF SUBEXCAVATION BELOW SUBGRADE WILL BE REQUIRED IN AREAS AS SHOWN IN THE CROSS SECTIONS. SUBEXCAVATION SHALL BE PAID FOR AS EMBANKMENT MATERIAL COMPLETE-IN-PLACE.

IN SOME AREAS, BLASTING AND/OR HEAVY EXCAVATION MAY BE REQUIRED. SEE DRAWING EM-1, LOGS OF EXPLORATORY HOLES, FOR SOILS TYPES. THE COST FOR BLASTING AND/OR HEAVY EXCAVATION WILL NOT BE PAID FOR SEPARATELY, BUT SHALL BE CONSIDERED SUBSIDIARY TO THE EARTHWORK OPERATION.

CONCRETE PIPE JOINT FASTENERS AS SHOWN ON M STANDARD ARE REQUIRED ON ALL CONCRETE CULVERT INSTALLATION, EXCLUDING SIDE DRAINS.

THE CONTRACTOR SHALL LIMIT CONSTRUCTION ACTIVITIES TO THOSE AREAS WITHIN THE LIMITS OF DISTURBANCE AND/OR TOES OF SLOPE AS SHOWN ON THE PLANS AND CROSS-SECTIONS. ANY DISTURBANCE BEYOND THESE LIMITS SHALL BE RESTORED TO ORIGINAL CONDITION BY THE CONTRACTOR AT HIS OWN EXPENSE. CONSTRUCTION ACTIVITIES, IN ADDITION TO NORMAL CONSTRUCTION PROCEDURES, SHALL INCLUDE THE PARKING OF VEHICLES OR EQUIPMENT, DISPOSAL OF LITTER, AND ANY OTHER ACTION WHICH WOULD ALTER EXISTING CONDITIONS.

FOR PRELIMINARY QUANTITIES OF PAVEMENT MATERIALS, THE FOLLOWING RATES OF APPLICATION WERE USED:

1. HOT BITUMINOUS PAVEMENT (GRADING E) AT 110 LBS PER SQ. YD./INCH
2. DILUTED EMULSIFIED ASPHALT (SLOW-SETTING) FOR TACK COAT AT 0.10 GAL. PER SQ. YD. CONSISTING OF ONE PART EMULSIFIED ASPHALT AND ONE PART WATER.
3. RATES OF APPLICATION SHALL BE DETERMINED BY THE ENGINEER AT THE TIME OF APPLICATION.

WHEN ORDERED BY THE ENGINEER, A TACK COAT OF EMULSIFIED ASPHALT (SLOW-SETTING) IS TO BE APPLIED BETWEEN PAVEMENT COURSES TO IMPROVE THE BOND. TACK COAT, IF REQUIRED, IS TO BE PAID FOR AS EMULSIFIED ASPHALT (SLOW-SETTING).

THE FOLLOWING SHALL BE FURNISHED WITH EACH BITUMINOUS PAVEMENT:

1. A SKI TYPE DEVICE AT LEAST 30 FEET IN LENGTH.
2. MIN. OF 1500 FEET OF CONTROL LINE AND STAKES.
3. NEXT TO FINAL LIFT SHALL BE WIRE CONTROLLED ON BELLEVUE. ANY LAYER OF BITUMINOUS PAVEMENT THAT IS TO HAVE A SUCCEEDING LAYER PLACED THEREON SHALL BE COMPLETED FULL WIDTH BEFORE SUCCEEDING LAYER IS PLACED.

CONTRACTOR SHALL ALSO BE RESPONSIBLE FOR RESETTING ALL LAND MONUMENTS DISTURBED BY CONSTRUCTION ACTIVITY. THE WORK SHALL BE DONE UNDER THE DIRECT SUPERVISION OF A REGISTERED LAND SURVEYOR. THE CONTRACTOR SHALL FILE MONUMENT RECORDS WITH THE BOARD OF REGISTRATION OF PROFESSIONAL ENGINEERS AND LAND SURVEYORS FOR ALL MONUMENTS RESTORED OR RESET. THIS WORK SHALL BE INCLUDED IN THE COST OF CONSTRUCTION SURVEYING.

FENCE ALONG THE RIGHT-OF-WAY, AS SHOWN ON THE PLANS SHALL BE CONSTRUCTED PRIOR TO START OF ANY CONSTRUCTION. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO BE SURE THAT LIVESTOCK ARE PROTECTED AND KEPT OUT OF THE RIGHT-OF-WAY PRIOR TO CONSTRUCTION.

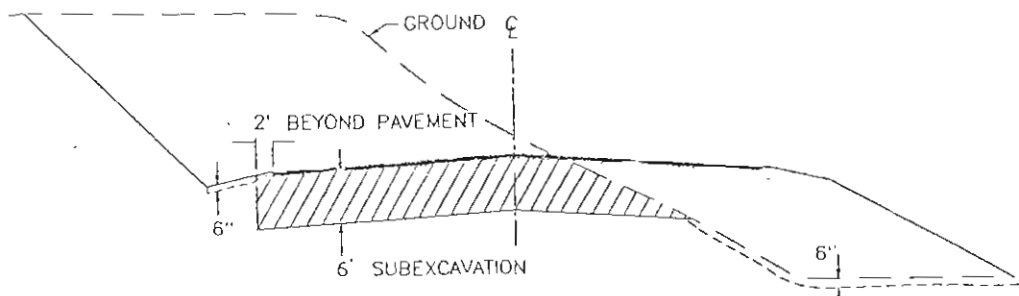
UTILITY LINES, AS SHOWN ON THIS PLAN AND PROFILE SHEETS, ARE PLOTTED FROM THE BEST AVAILABLE INFORMATION. THE CONTRACTOR'S ATTENTION IS DIRECTED TO PARAGRAPH 107.10 OF THE STANDARD SPECIFICATIONS CONCERNING UTILITIES. THE CONTRACTOR WILL CALL 534-6700 FOR UTILITY LOCATIONS 48 HOURS PRIOR TO DIGGING.

THE CONTRACTOR SHALL INITIATE A REQUEST TO PUBLIC SERVICE COMPANY FOR ANY CONSTRUCTION-RELATED TEMPORARY ELECTRICAL POWER SOURCES AS SOON AS POSSIBLE. IN SOME INSTANCES, UP TO 30 DAYS MAY BE REQUIRED TO PROVIDE THE SOURCES. THE REQUEST IS TO BE PROCESSED THROUGH PUBLIC SERVICE COMPANY OF COLORADO--LIGHTING UTILIZATION AT (303) 571-7537.

THE CONTRACTOR MUST MAINTAIN PRESENT ACCESS TO PROPERTY OWNERS AT ALL TIMES DURING CONSTRUCTION.

ALL EXPOSED CONCRETE SURFACES EXCEPT CURB AND GUTTER PLACED ON THIS PROJECT WILL REQUIRE A COLORED CONCRETE COATING OF EITHER (FEDERAL STANDARD 595 A) FEDERAL COLOR NO. 30219 OR 31667 AS DESIGNATED BY THE ENGINEER. THIS WORK WILL BE SUBSIDIARY TO THE CONCRETE PLACEMENT.

SUBEXCAVATION FOR CROWNED SECTIONS



SUBEXCAVATION NOTES:
DEPTH OF SUBEXCAVATION
MAY BE CHANGED AS
DETERMINED BY THE ENGINEER

LEGEND

SUBEXCAVATION 
COMPACTION FOR
BASE OF CUTS
AND FILLS 

LIMITS OF SUBEXCAVATION		
BASELINE	BEGIN STATION	ENDING STATION
QUINCY AVENUE	117+18.4	118+00.0

IT IS ESTIMATED THAT 2 DELINEATORS (TYPE 1) CRYSTAL AND 15 DELINEATORS (TYPE 11) 2 1/2\"/>

IT IS ESTIMATED THAT 20 PROSION SALES FOR TEMPORARY DRAINAGE WILL BE REQUIRED ON THIS PROJECT.

IT IS ESTIMATED THAT 100 LIN. FT. OF DIRECT-BURIAL CABLE WILL BE REQUIRED FOR ELECTRICAL SERVICE CONNECTION ON THIS PROJECT.

IT IS ESTIMATED THAT 100 LIN. FT. OF PAVEMENT MARKING TAPE WILL BE REQUIRED FOR THIS PROJECT.

THE GAS LINE AT STA. 117+70 WILL BE RELOCATED BY PUBLIC SERVICE FORCES, AT NO COST TO THE PROJECT.

THE ENTIRE EMBANKMENT FOR THE BIKEPATH NEAR ELDRIDGE WILL BE CONSTRUCTED TO SUBGRADE AS SOON AS PARCEL NO. 321 IS AVAILABLE.

EARTHWORK ON WEST TURKEY CREEK FRONTAGE ROAD AND QUINCY AVE. (INCLUDING THE AREAS NEAR THE STRUCTURE) MUST BE BUILT TO WITHIN 6\"/>

AFTER QUINCY STRUCTURE BOTTOM DECK IS IN PLACE AND HAS ACHIEVED ENOUGH STRENGTH TO SUPPORT THE UTILITY TREES, 4 DAYS OF UTILITY CONDUIT PLACEMENT WILL TAKE PLACE AND MUST BE PROTECTED FROM DAMAGE DUE TO CONSTRUCTION.

CALL UTILITY NOTIFICATION
CENTER OF COLORADO
1-800-922-1987
OR 534-6700 IN METRO DENVER
CALL 2-BUSINESS DAYS IN ADVANCE
BEFORE YOU DIG, GRADE OR EXCAVATE
FOR THE MARKING OF UNDERGROUND
MEMBER UTILITIES.

FINAL
SUMMARY OF APPROXIMATE QUANTITIES

AS CONSTRUCTED			FED. ROAD REGION	DIVISION	PROJ. NO.	SHEET NO.
NO REVISIONS	REVISED	VOID	VIII	COLO.	IXFU 470-1(55)	6

INDEX			CONTRACT ITEM NO.	CONTRACT ITEM	UNIT	ROADWAY		STR. NO. F-16-NE								PROJECT TOTALS		DIFF. +/-	% PLAN
						PLAN	AS CONST.	PLAN	AS CONST.							PLAN	AS CONST.		
3	1		201	CLEARING AND GRUBBING	L S	0.5	0.5									0.5	0.5	—	100
3	2		202	REMOVAL OF STRUCTURES AND OBSTRUCTIONS	L S	0.5	0.5									0.5	0.5	—	100
3	3		202	REMOVAL OF FENCE	LIN FT	715	687									715	687	-28	96
3	4		203	EMBANKMENT MATERIAL (COMPLETE IN PLACE)	CU YD	38215	38215									38215	38215	—	100
3	5		206	STRUCTURE EXCAVATION	CU YD			1271	1275							1271	1275	+4	100.3
3	6		206	STRUCTURE BACKFILL (CLASS 1) DELETED BY CDOH FORM 105	CU YD	114	0	203	0							317	0	-317	0
3	7		206	STRUCTURE BACKFILL (CLASS 2)	CU YD			330	647							330	647	+317	196
3	8		206	FILTER MATERIAL (CLASS B)	CU YD	1037	1037									1037	1037	—	100
3	9		502	DRILLING HOLE TO FACILITATE PILE DRIVING CMO #2	LIN FT			480	0							480	0	-480	0
3	39		502	FACILITATE PILE DRIVING CMO #2	LS			0	1.0							0	1.0	-1.0	—
3	10		502	STEEL PILING (HP 12X74)	LIN FT			1085	1,079							1085	1,079	-6	100
3	11		506	IMPERVIOUS PLASTIC LINING	SQ YD	446	513									446	513	+67	115
3	12		507	CONCRETE SLOPE AND DITCH PAVING (REINFORCED)	CU YD			148	152.36							148	152.36	+4.36	103
3	13		515	WATERPROOFING (MEMBRANE)	SQ YD			582	581							582	581	-1	99.8
3	14		518	BRIDGE COMPRESSION JOINT SEALER	LIN FT			72	66							72	66	-6	92
3	15		518	WATERSTOP (6 INCH)	LIN FT			34	34							34	34	—	100
3	16		601	CONCRETE CLASS A (BRIDGE)	CU YD			487	519.78							487	519.78	+32.78	107
3	17		601	CONCRETE CLASS A (WALL)	CU YD			161	161.00							161	161.00	—	100
3	18		601	CONCRETE CLASS S (BRIDGE)	CU YD			2304	2304.88							2304	2304.88	+0.88	100.03
3	19		602	REINFORCING STEEL	LB			10834	10,834							10834	10,834	—	100
3	20		602	REINFORCING STEEL (EPOXY COATED)	LB			460568	480,073							460568	480,073	+19,505	104
3	21		605	6 INCH NON-PERFORATED PIPE UNDERDRAIN	LIN FT	20	78									20	78	+58	390
3	22		605	6 INCH PERFORATED PIPE UNDERDRAIN	LIN FT	180	180									180	180	—	100
3	23		607	END POST DELETED BY CDOH FORM #105	EACH	6	0									6	0	-6	0
3	24		607	CORNER AND LINE BRACE POST	EACH	3	1									3	1	-2	33
3	25		607	FENCE BARBED WIRE WITH METAL POSTS DELETED BY CDOH FORM 105	LIN FT	360	0									360	0	-360	0
3	26		607	FENCE COMBINATION WIRE WITH TREATED WOODEN POSTS	LIN FT	1005	805									1005	805	-200	80
3	27		607	FENCE CHAIN LINK SPECIAL	LIN FT			761	774							761	774	+13	102
3	28		613	1-1/2 INCH ELECTRICAL CONDUIT	LIN FT			66	74							66	74	+8	112
3	29		613	2 INCH ELECTRICAL CONDUIT (PLASTIC)	LIN FT			378	408							378	408	+30	108
3	30		613	3 INCH ELECTRICAL CONDUIT (PLASTIC)	LIN FT			378	411							378	411	+33	109
3			613	DIRECT BURIAL CABLE	LIN FT	—100—										—100—			
3	31		613	WIRING	L S	1	1									1	1	—	100
3	32		613	LUMINAIRE HIGH PRESSURE SODIUM (WALL TYPE) (16,000 LUMEN)	EACH			2	2							2	2	—	100

NO REVISIONS

REVISÉD 7-15-21

void

. PROJ. NO.

viii

330

IXFU 470-1(55)

SHEET
NO

7-

1-JUL-1988 USER\$DISK:(DRAW,PJ.87035.ROWY.H103.TAB)V113FB

STRUCTURE QUANTITIES

AS CONSTRUCTED			FED. ROAD REGION	DIVISION	PROJ. NO.	SHEET NO.	TOTAL SHEETS		
NO REVISIONS	☐	REVISED <u>7-15-67</u>	☐	VOID ☐	VIII	CLD.	IX FU 470-1(55)	8	49

[illegible]

* FROM BRIDGE ABUTMENT DETAIL

AS CONSTRUCTED				FED ROAD RECU	DIVISION	PROJ NO	SHEET NO.	SHEET TOTALS
NO REVISIONS		REVIS D	7-15-87	VIII	COLL.	IXFU 470-11551	9	49

TABULATION OF REMOVALS

STATION TO STATION	SIDE	REMOVAL OF PIPE	REMOVAL OF WOOD POST	REMOVAL OF ASPH. F. NAT	REMOVAL OF FENCE	REMOVAL OF G. C.	REMOVAL OF STRUCTURES AND OBSTRUCTIONS	REMARKS
		LIN. FT.	EACH	SQ. YD.	LIN. FT.	EACH	LUMP SUM †	
					FINAL		FINAL	
<u>QUINCY AVE.</u>								
112+10 TO 115+00	LT./RT.				525	587		
112+42 TO 112+95	RT.				190	100		
112+70	RT.							BARN
TOTALS					715		1	2.5

REMOVAL OF G. C. IS TO BE DONE BY THE CONTRACTOR AT HIS OWN RISK AND EXPENSE.

THIS ITEM WILL INCLUDE ALL OTHER VISIBLE OBSTRUCTIONS NOT SPECIFICALLY LISTED IN THIS TABULATION.

TABULATION OF RESET ITEMS

STATION TO STATION	SIDE	RESET IMPACT ATTENUATOR	RESET WATER SERVICE	ADJUST VALVE BOX
		EACH	EACH	EACH
BELLEVIEW AVE				
8+	RT.		1	1
9+15	MED.	1		
10+85	MED.	1		
TOTALS		2	1	1

WITH CATCH BASIN TO PROTECT FROM SPILLAGE OF HIGHWAY MATERIALS. VARY AT 2000 - 5000 (SEE DETAILS)

STATION TO STATION	SOURCE	PLANT MIX SEAL COAT (TYPE B) (HAUL AND ASPHALT)		HOT BITUMINOUS PAVEMENT (GR. 5) (HAUL AND ASPHALT)					
		3/4"	3"	2"	2"	2"	2"	2"	2"
		TONS	FINAL	TONS	FINAL	TONS	FINAL	TONS	FINAL
BELLEVIEW AVE. (MAINLINE)									
6+00 TO 8+00		39		44		27		27	
8+00 TO 8+75		20		84		55		55	
8+75 TO 12+50		77		26		284		284	
12+50 TO 15+00		49		202		131		131	
15+00 TO 16+00		15							
BELLEVIEW DETOUR IRREGULARITIES 10A				128		86		86	
				89					
TOTALS		230		973		583		583	

IT IS ESTIMATED THAT 709 GALLONS OF EMULSIFIED ASPHALT (SLOW-SETTING) FOR TACK COAT WILL BE REQUIRED.

IT IS ESTIMATED THAT 100 TONS OF HOT BITUMINOUS PAVEMENT (PATCHING) WILL BE REQUIRED.

STABILIZATION BASED ON: (BELLEVIEW)

20 YEAR DESIGN - 18 ^k ESAL	226,000
"R" VALUE	7
SERVICEABILITY INDEX	2.5
REGIONAL FACTOR	1.75
STRENGTH COEFFICIENT (H.B.P. GR. EX)	0.44
(P.M.S.C.)	0.25
S.N.	3.30

TABULATION OF FENCING

STATION	SIDE	END POST		FENCE COMBINATION WIRE W/TREAT WOODEN POSTS		FENCE BARBED WIRE WITH METAL POSTS		FENCE COMBINATION WIRE W/TREAT METAL POSTS		CORNER AND LINE BRACE POSTS		10 FOOT GAL.		24 FOOT GAL.	
		EACH	FINAL	LIN. FT.	FINAL	LIN. FT.	FINAL	LIN. FT.	FINAL	EACH	FINAL	EACH	FINAL	EACH	FINAL
QUINCY															
109+05	LT/RT					260	0			2	0				
109+05 TO 113+00	RT.	1	0	395	395										
109+05 TO 112+05	LT.	1	0	410	410					1	1				
113+00 TO 115+00	LT.	2	0	200	0										
117+10 TO 117+60	RT.	2	0			100	0								
BELLEVIEW															
6+00 TO 8+30															
12+00 TO 13+15															
13+35 TO 15+00															
13+25 TO 15+00															
TOTALS		6	0	1005	805	360	0			3	1				

0 CONNECT TO EXISTING FENCE

0

NOTE: COST OF CONNECTING NEW FENCE TO EXISTING FENCE OR STRUCTURE IS TO BE INCLUDED IN THE WORK

SEE M-607-1 "SPECIAL FOR THIS PROJECT" 39 INCH MESH REQUIRED FOR THIS PROJECT

SUMMARY OF EARTHWORK

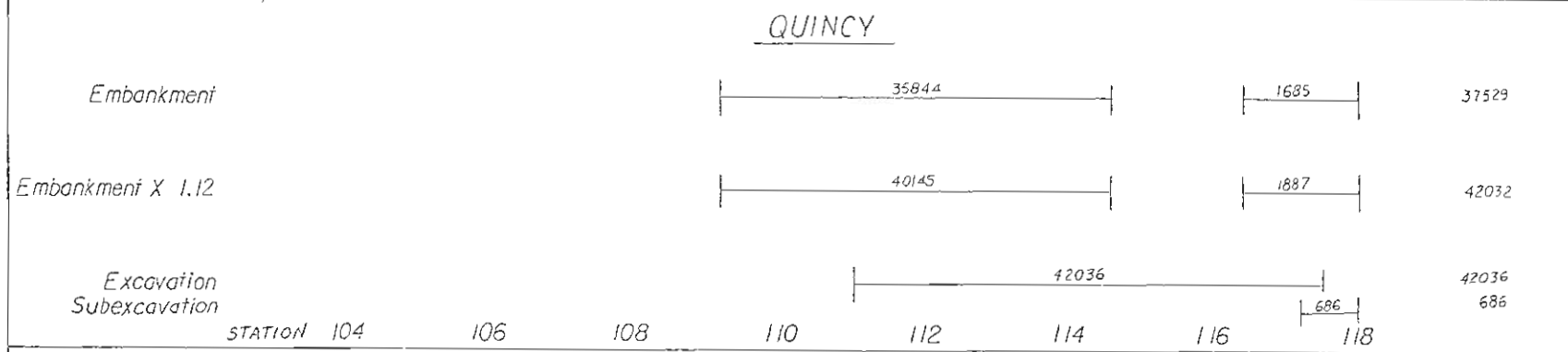
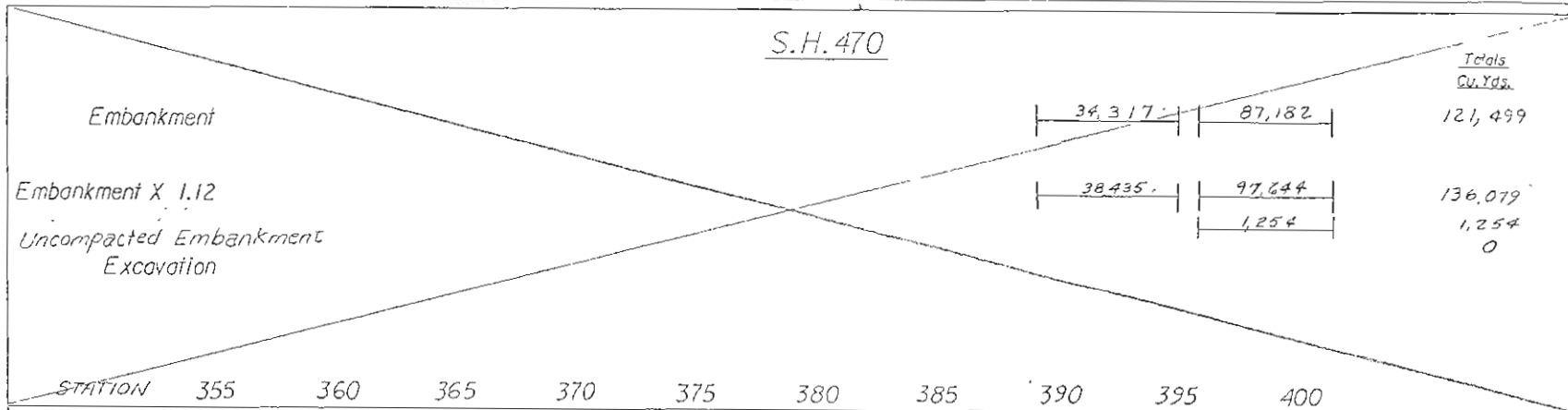
INDEX			ITEM	PROJECT TOTALS ROADWAY	
BOOK	PAGE	SHEET		CU. YD.	FINAL
			<u>Embankment Material (CJP)</u>		
			S.H. 470 (From Computer Cross Sections)	122,753	
			Bellevue & Detour (From Cross Sections)	4,372	
			QUINCY (From Cross Sections)	37,529	37,529
			QUINCY (TO REPLACE SUBEXCAVATION)	686	686
			From Structure Quantities as Embankment	270	
			Total For Pay Quantities	38,215	38,215
			(For Information Only)		
			Roadways Quantities Balance		
			Embankment Material (From Cross Sections) X Factor		
			38,215 X 1.12	42,801	42,801
			EXCESS EXCAVATION	821*	821
			Total For Balance	43,622	43,622
			(For Information Only)		
			Unclassified Excavation		
			S.H. 470 (From Computer Cross Sections)	0	0
			Estimated For Cut Slope Treatment	900	900
			Bellevue & Detour (From Cross Sections)	4,917	
			QUINCY (FROM CROSS SECTIONS)	42,036	42,036
			QUINCY (SUB-EXCAVATION)	686	686
			From Structure Quantities	4,230	
			SW Ramp	137,266	
			(Minimum Soil Support R-Value shall be 7)		
			Total For Balance	43,622	43,622

MOISTURE DENSITY CONTROL
(For Information Only)

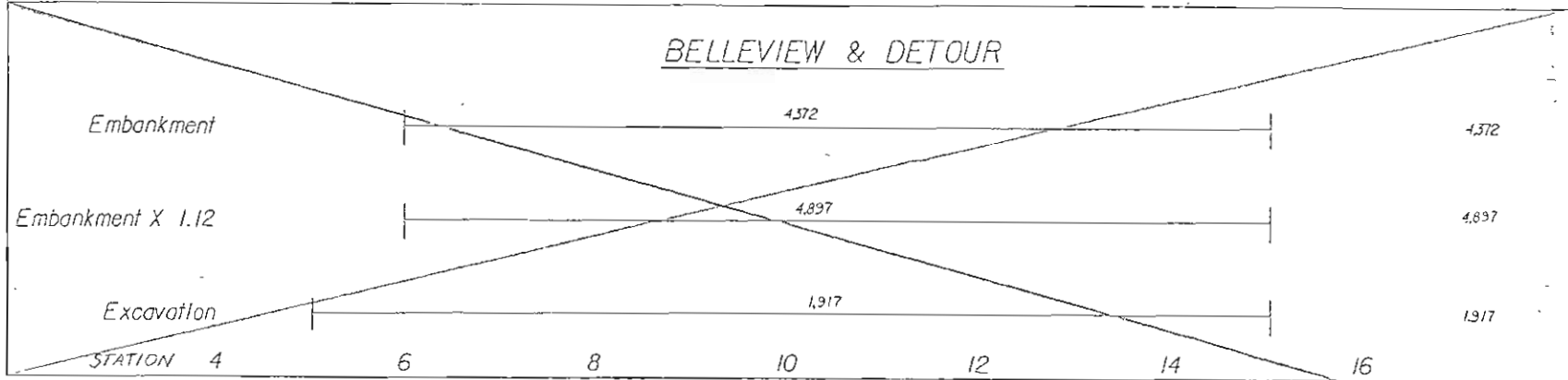
ITEM	CU. YD.	
COMPACTION (AASHTO T-99)		
- EMBANKMENT MATERIAL	138,215	38,215
- BASES OF CUT & FILLS	7,662	1662
TOTAL	39,877	39,877
WETTING		
- ROADWAY (39,877 X 0.04)	1,595	1595
- DUST PALLIATIVE	1050	50
TOTAL	1,645	1645

AS CONSTRUCTED			FED. ROAD REGION	DIVISION	PROJ. NO.	SHEET NO.	SHEET TOTALS
NO REVISIONS ☐ REVISED ☒ VOID ☐			VIII	COLO.	IXFU470-K(55)	10	49

SOURCE OF ROADWAY QUANTITIES

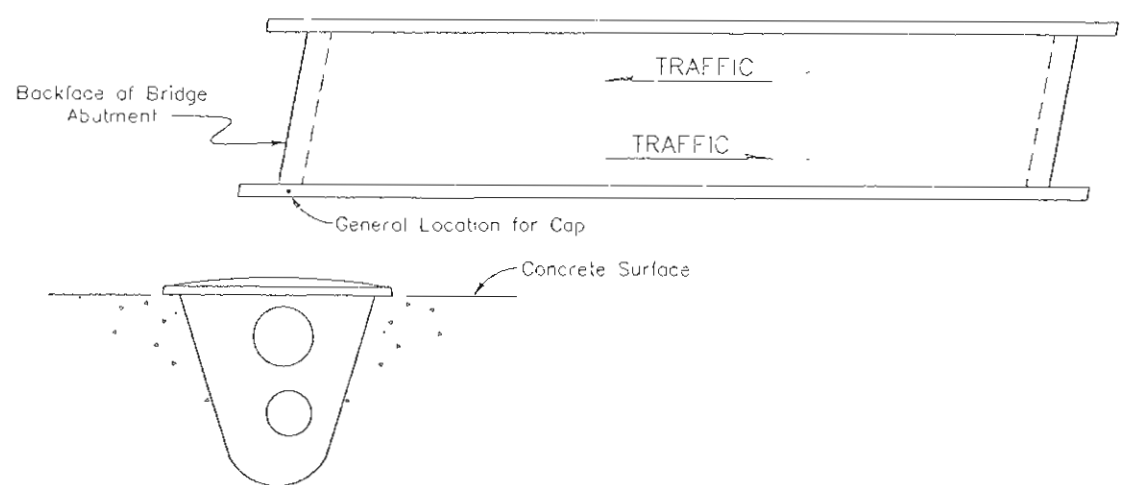


* UNCOMPACTED, to be used to steepen ML fill slopes Sta 389+52 to 392+70 or as directed by the Engineer

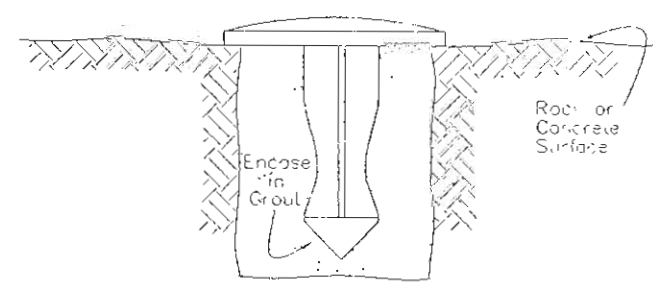


NOTE: Embankment for this project will be generated between Future Ramp Stations 1120+00 to 1133+00 for Summary of Earthwork Quantity purposes. The contractor may elect to obtain material from elsewhere on these projects with the engineer's approval.

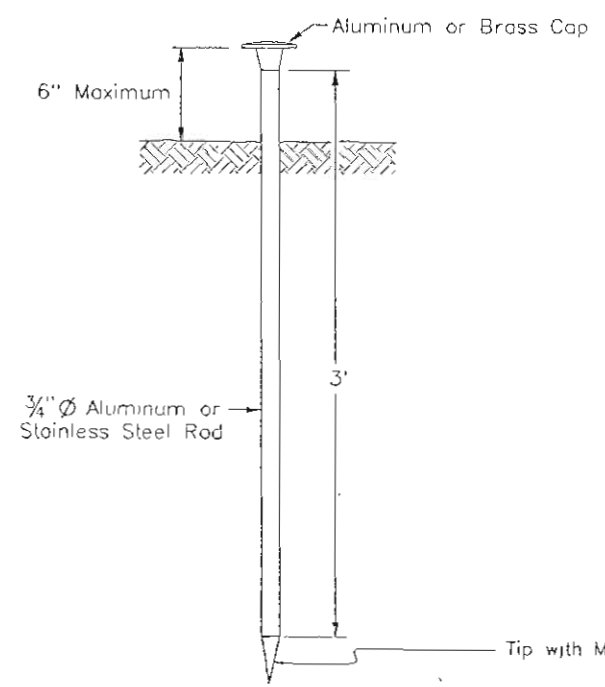
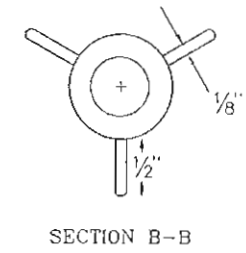
AS CONSTRUCTED			FED. ROAD DIST. NO.	DIVISION	PROJ. NO.	SHEET NO.	
NO REVISIONS	7-11-87	REVISED	VOID	VI	COLO.	2-1-110-1(55)	11



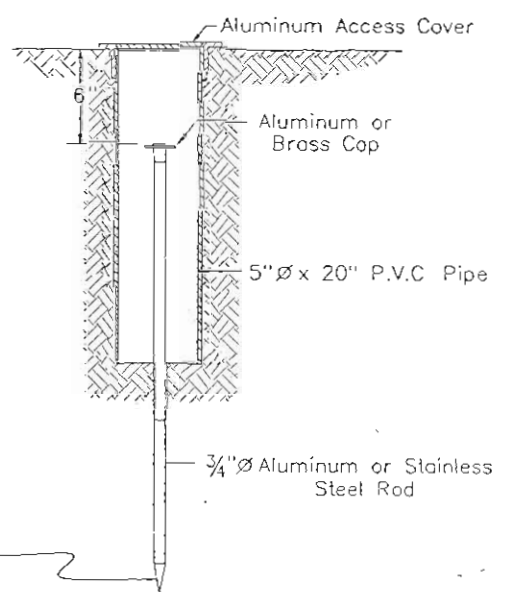
TYPE 4 MONUMENT



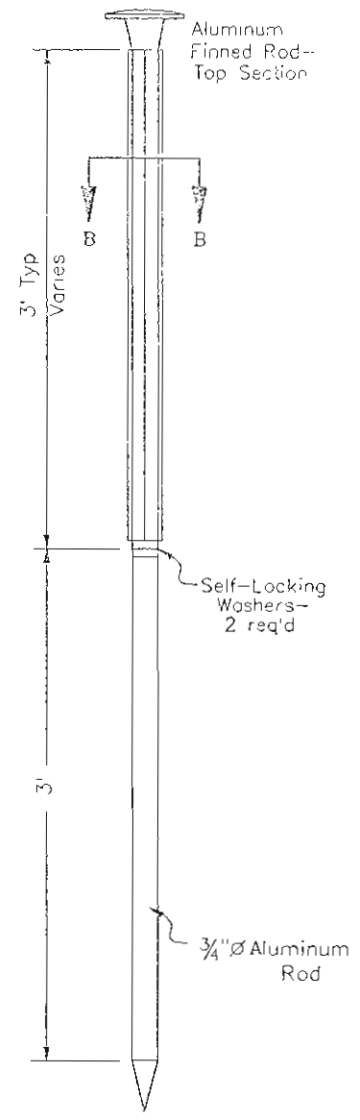
TYPE 5 MONUMENT



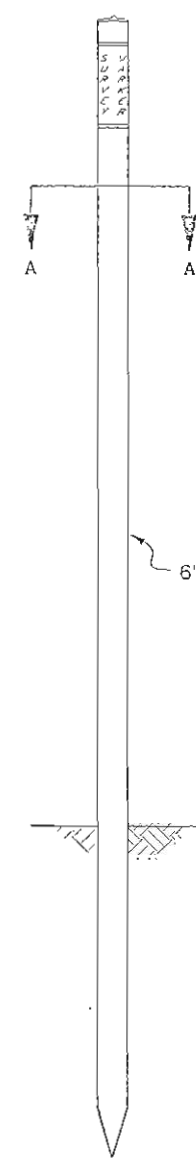
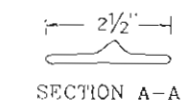
TYPE 1 MONUMENT



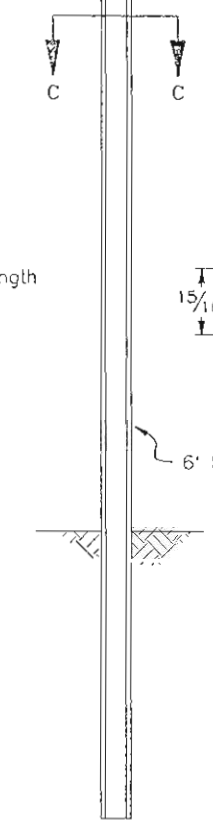
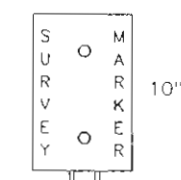
TYPE 1A MONUMENT & TYPE 2A MONUMENT
Includes Monument Box



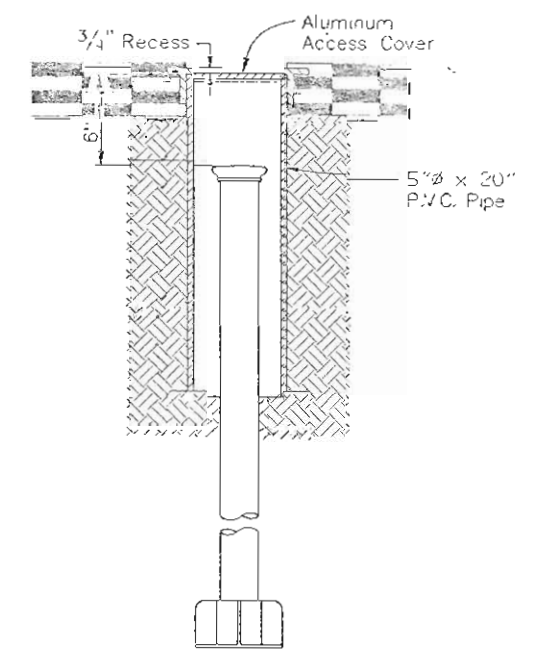
TYPE 2 MONUMENT



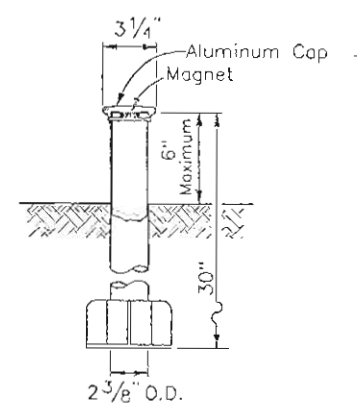
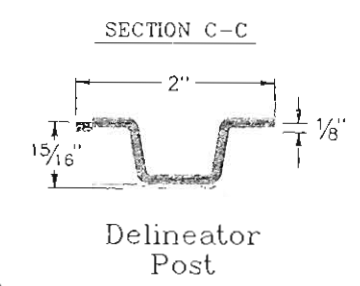
Metal Sign Panel



WITNESS POSTS



TYPE 3A MONUMENT,
Roadway Installation



TYPE 3 MONUMENT

SURVEY
MONUMENTS



REFERENCE MONUMENT CAP



ROW MONUMENT CAP

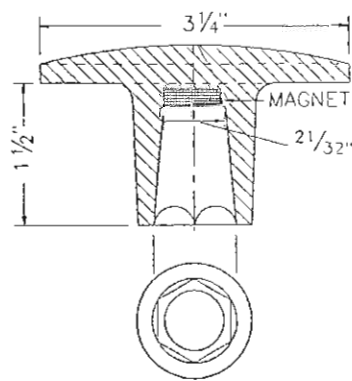


CONTROL MONUMENT CAP

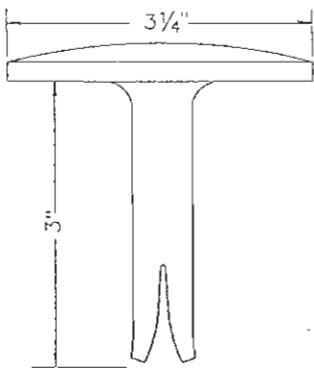


ALIQUT CORNER CAP

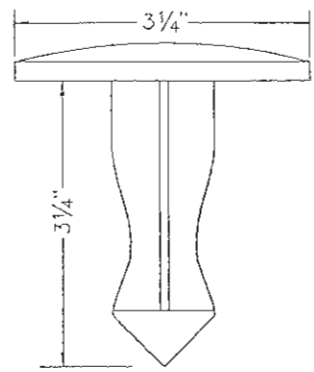
NOTE: (A BLANK CAP MAY BE SUBSTITUTED IF ANY ABOVE CAPS ARE NOT AVAILABLE)



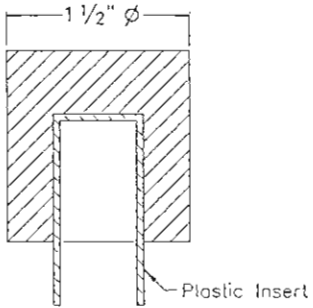
BRASS AND ALUMINUM CAPS
BRASS CAP USED WITH STAINLESS STEEL
ROD ALUMINUM CAP USED WITH ALUMINUM ROD
(Threaded Cap is also available)



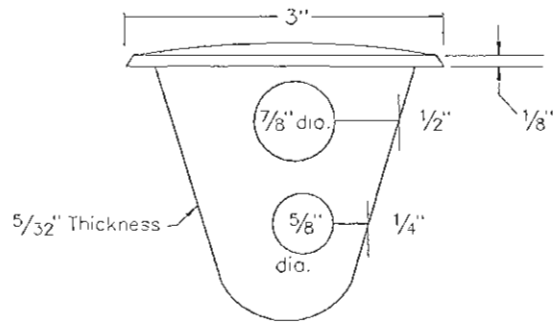
BRASS CAP
FOR PLACING IN EXISTING
CONCRETE OR ROCK



BRASS CAP
FOR PLACING IN EXISTING
CONCRETE OR ROCK



TEMPORARY
ALUMINUM CAP



BRASS CAP
INSTALL DURING PLACEMENT OF CONCRETE.
PLACE A REINFORCING BAR THRU ONE HOLE.

MONUMENT APPLICATION

CAP TYPE	MONUMENT TYPE							
	1	1A	2	2A	3	3A	4	5
REFERENCE	X						X	
ROW	X							X
CONTROL		X	X	X			X	
ALIQUT CORNER					X	X		
WITNESS POST (WHEN REQUIRED)	X	X	X	X	X			X

AS CONSTRUCTED	FED. ROAD DIVISION	DIVISION	PROJ. NO.	SHEET NO.
NO REVISIONS 7-12-84	7-12-84	REVISD	VOID	12

TYPE 1 AND TYPE 1A ALUMINUM AND STAINLESS STEEL DRIVEN MONUMENTS

Monuments are assembled by attaching the proper size tip to one end of a section of rod and the 3" long x 3/4" dia stainless steel driver to the other end. A steel driver is then placed over the stainless steel driver for the hammer to contact. When additional sections of rod are needed, they are to be attached using a set of self-locking washers. Rods are stocked in two foot and three foot sections. One foot sections may be obtained by cutting a two foot section in half. The rod section is driven by using the proper hammer with a soft face. A power adapter is available for use with a gasoline powered driver.

Type 1 monuments typically use a three foot section of rod. When subsurface rock or concrete is encountered less than 3 feet below the ground surface, the rod shall be embedded in the rock or concrete at least 6" with a grout material. The rod may be shortened to accommodate the conditions.

Type 1A monuments may be driven to refusal by threading additional sections onto the first section. Added sections require a set of lock washers. Refusal is defined as one inch of movement with four blows of a 12 lb. hammer. A driven cap is necessary (vs. a threaded cap) when a cut rod is used. The soft faced hammer is also required to seal the driven caps onto the rod.

TYPE 2 AND TYPE 2A ALUMINUM FINNED ROD MONUMENTS

A Type 2 monument is used for horizontal control monuments where loose material is encountered. The larger size tip is to be used on one end of the three foot rod and the fins on the opposite end of the three foot rod are to be bent over to accomodate the cap. If more vertical stability is required for use as a vertical control monument, the added sections which are not finned, should be driven first. The number of sections to be added will vary with needs and type of soil.

Type 1A monument note above also applies to Type 2A monument.

TYPE 3 AND TYPE 3A ALUMINUM PIPE MONUMENTS

This monument is used to monument an aliquot corner. The installation of this monument and record filing shall be done in accordance with the Colorado Revised Statutes, Title 38, Articles 50 and 51. Also refer to the CDOH Survey Manual. The Land Surveyor's license number and the year shall be stamped on the cap.

TYPE 4 BRASS CAP MONUMENT

This monument is placed in concrete, such as bridge super structure concrete. It is placed above a footing support over the abutment or over a pier. An alternate placement is vertically in the abutment wall or pier if only an elevation reference is desired and it is possible to leave access for an elevation rod.

TYPE 5 BRASS CAP MONUMENT

This monument is placed in locations where a Type 1 or Type 1A monument cannot be placed. A common application is in a large boulder where a property corner has been placed. This monument could also be placed in a sidewalk.

WITNESS POSTS

The witness post, when required, will be supplied by the Division and installation shall be included in the work. It shall be driven within 10 feet of the monument, when possible. A delineator post with a 6" x 10" metal sign panel may be used in lieu of the plastic post. This post is to conform to Standard S-612-1.

SELF-LOCKING WASHERS

A set of two washers shall be used at rod joints and threaded cap joints, but not at tip joints.

TEMPORARY MONUMENTS

This cap is used on a #5 rebar.

SURVEY
MONUMENTS

AS CONSTRUCTED
NO REVISIONS 7-15-69 REVISED VOID

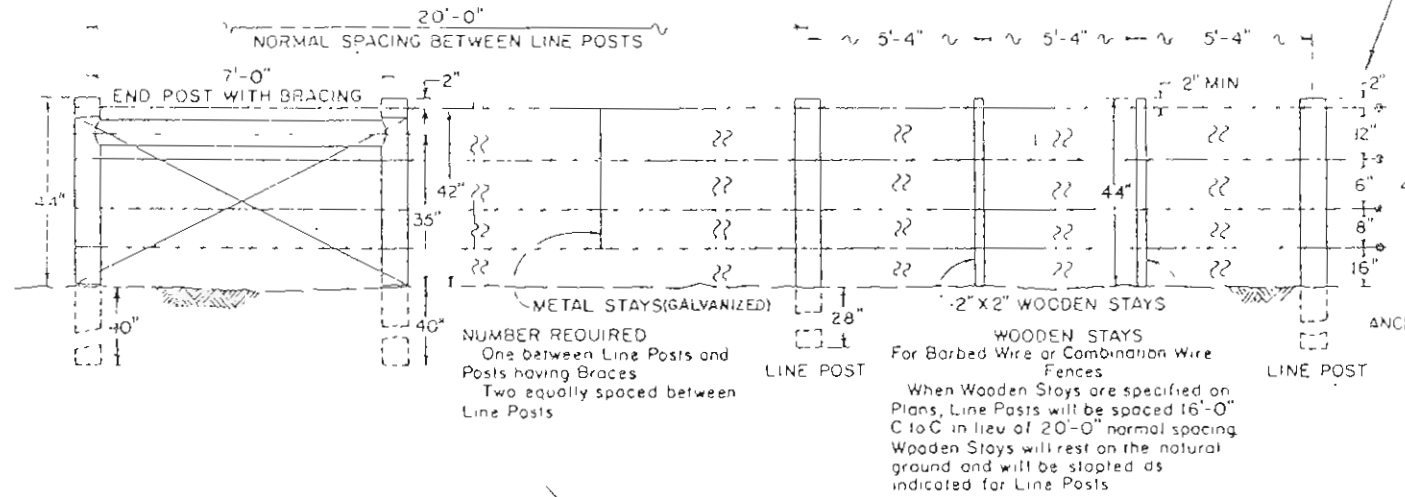
STANDARD 11-607-1

(SHEET 1 OF 2 SHEETS)
(JANUARY, 1967)

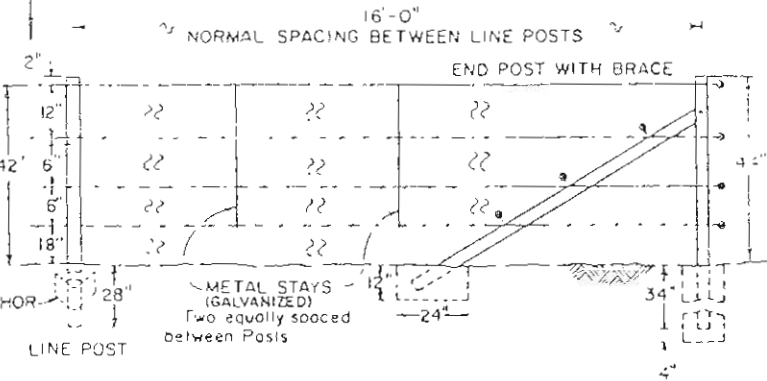
FED. ROAD REG. NO.	DIVISION	PROJECT NO.	SHEET NO.
VIII	COLORADO	1XFU 470-1(55)	13

REVISIONS

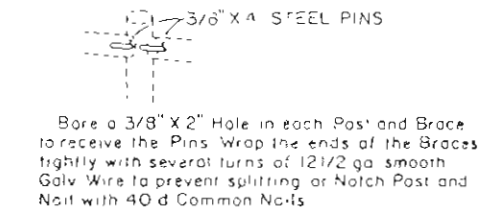
BARBED WIRE FENCE WITH WOODEN POSTS



BARBED WIRE FENCE WITH METAL POSTS (SPECIAL FOR THIS PROJECT)

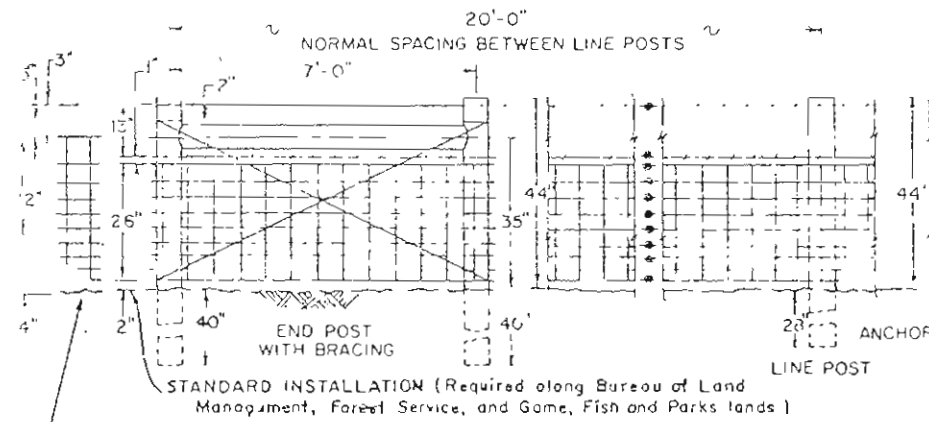


CROSS BRACE DOWELING DETAIL

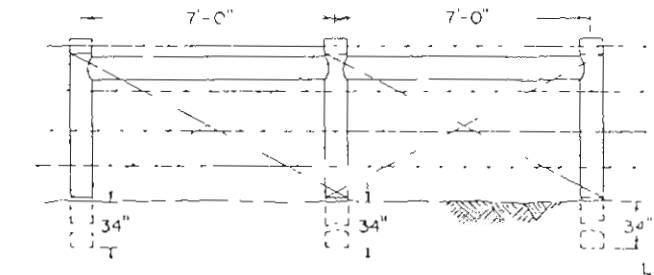
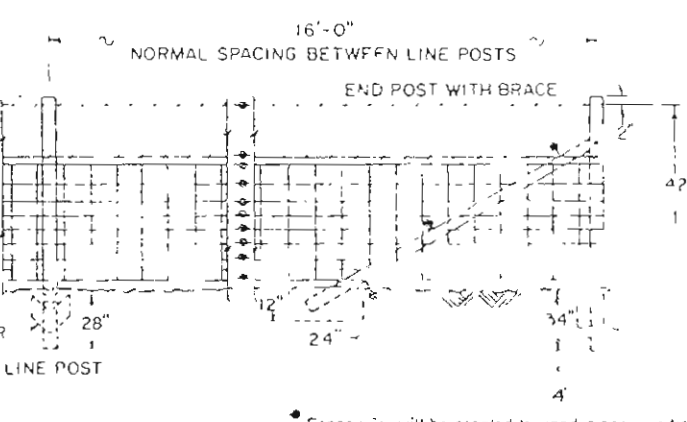


NOTE:
See Sheet 2
for General Notes.

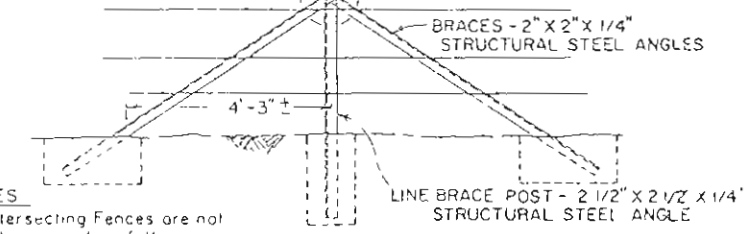
COMBINATION WIRE FENCE WITH WOODEN POSTS



COMBINATION WIRE FENCE WITH METAL POSTS

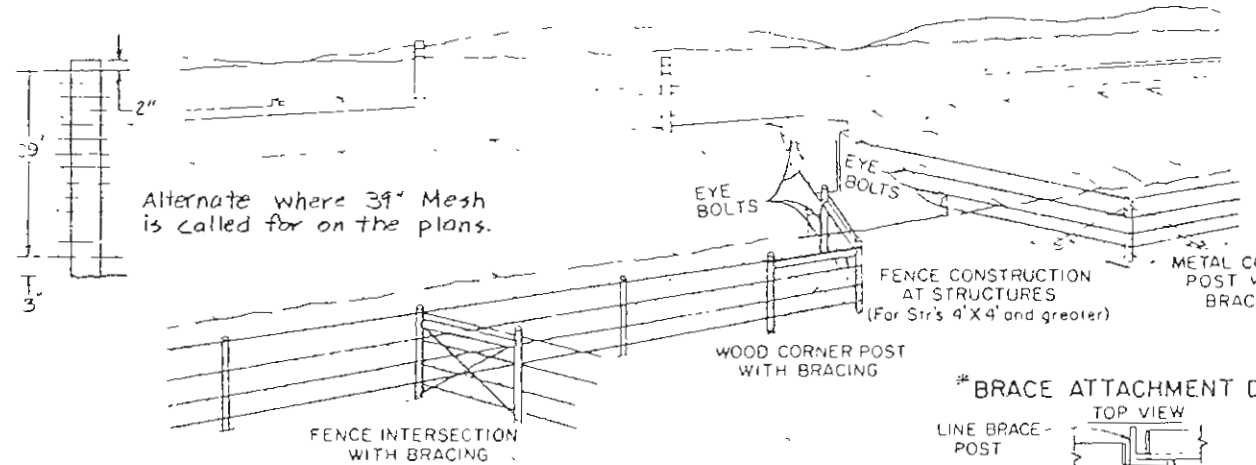


SEE BRACE ATTACHMENT DETAIL



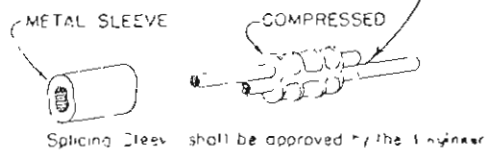
TYPICAL INSTALLATION AT FENCE INTERSECTIONS

ILLUSTRATIVE SKETCH SHOWING TYPICAL EXAMPLES FOR CONSTRUCTING FENCES

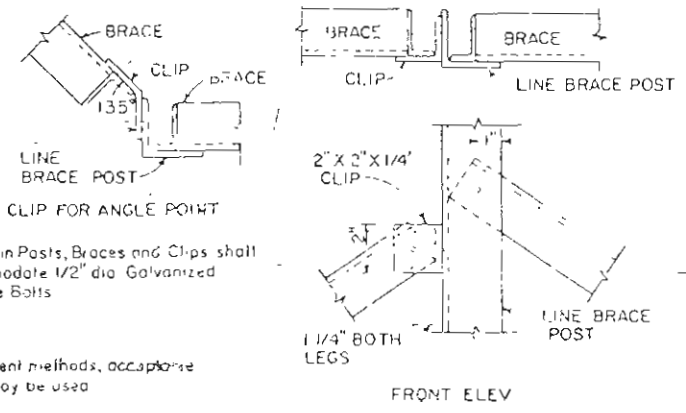


* Fence wire will be stapled to wooden posts or tied to metal posts as shown marked on barbed wire or combination wire fence details.

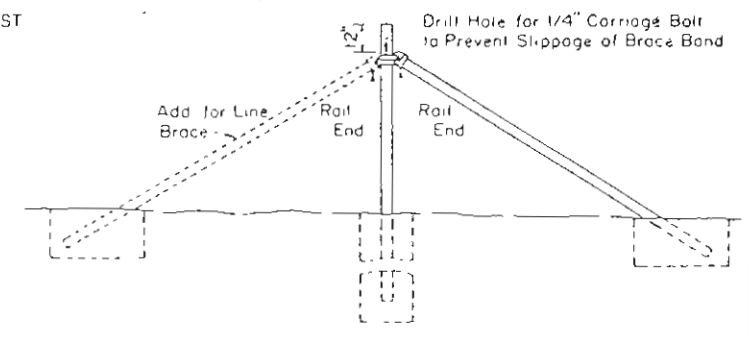
ACCEPTABLE WIRE SPlice



* ALTERNATE BRACE ATTACHMENT DETAIL



ALTERNATE POST (FOR END, CORNER OR LINE BRACE POSTS)



DEPARTMENT OF HIGHWAYS
STATE OF COLORADO
DIVISION OF HIGHWAYS

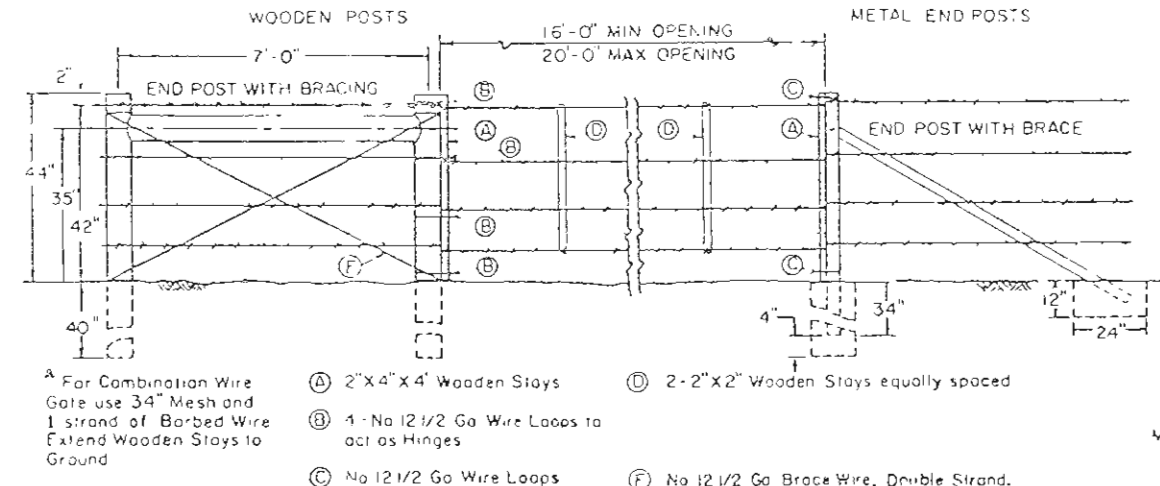
WIRE FENCES AND GATES

Designed by LEOI Approved by TEF
Made by TEF Staff Drawn by
Checked by EEO Date: July 1, 1965

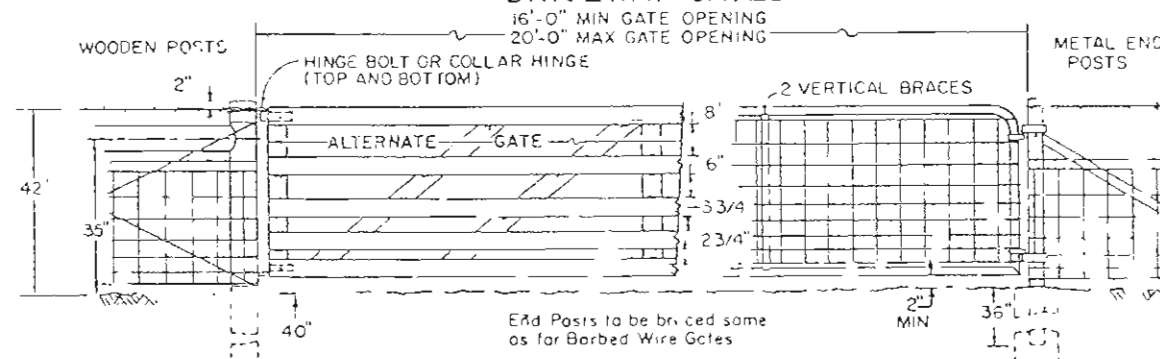
Alternate where 39" Mesh is called for on the plans.

NOTE:
At all structures of 4' x 4' and over, the fence shall be ended at eye-bolts in the wings of the structure. Where the type of structure prohibits the use of eye-bolts, an end post with brace shall be used. Eye-bolts shall be made of 1/2" round bars with a minimum of six (6) inches of body length embedded in the concrete (hooked or bent) and a minimum of 1" inside eye diameter. Eye-bolts shall be furnished and installed by the Contractor. Cost of eye bolts will be included in the contract price for fencing.

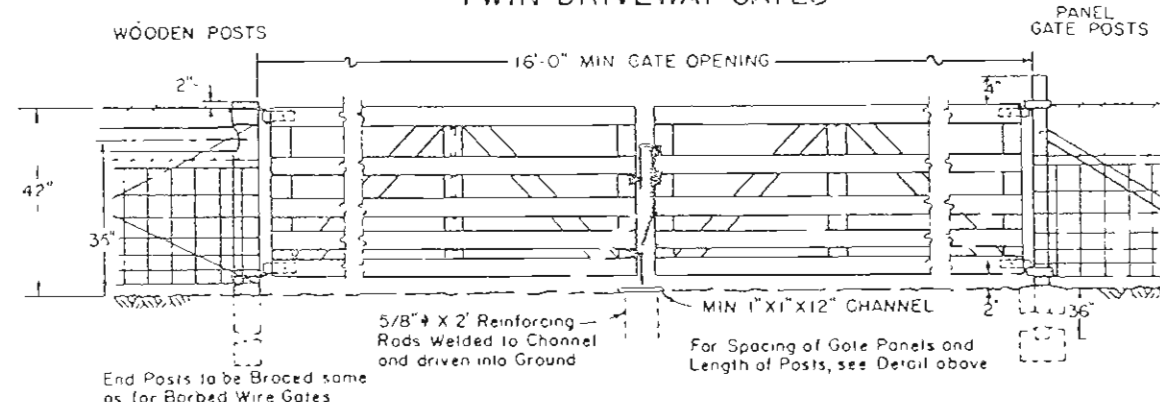
BARBED WIRE GATE



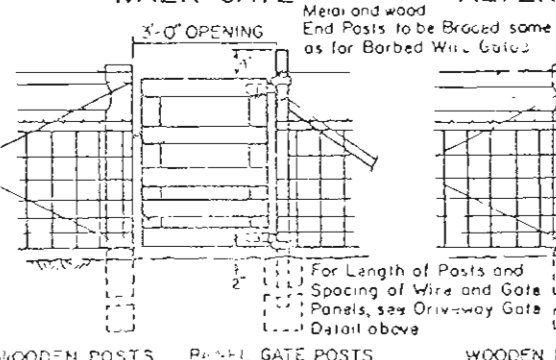
DRIVEWAY GATES



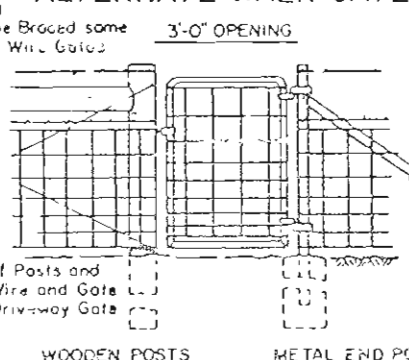
TWIN DRIVEWAY GATES



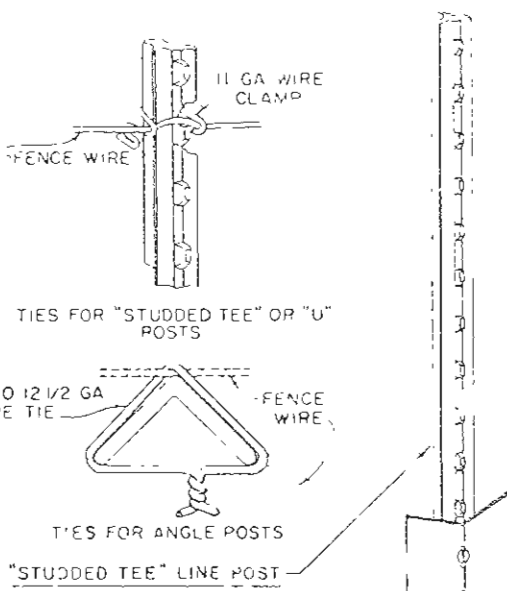
WALK GATE



ALTERNATE WALK GATE



FENCE WIRE TIES



GENERAL NOTES

All work shall be done in accordance with Standard Specifications applicable to the project and approved by the Engineer. Standard Specifications are hereby incorporated by reference.

Fence wire shall be sized, double wrapped and tied off at end posts, angle posts or line brace posts. Fence to be continued shall then be started in like manner.

Fence wire to be placed on either road or field side of posts depending on local conditions, i.e. no wire should be placed on the side of the post which would prevent tension on fence ties. This will also apply where wind drift, tumble weeds or other conditions would exert unusual pressure against the wire.

WOOD POSTS

All line posts shall have a min dia of four (4) inches and be 6'-0" long Min.

All end, corner, intersection and brace posts shall have a minimum diameter of five (5) inches and be 7'-0" long.

Fence wire will be stapled to wooden posts or tied to metal posts as shown marked on barbed wire or combination wire fence details. Staples shall be 3/8" wire, min at least 1/2" from end of post.

METAL POSTS

All posts and braces shall be of the types and weights shown or acceptable equivalents.

Holes to be provided in end, corner and gate posts as detailed.

CORNER AND LINE BRACE POSTS

Type - 2 1/2" X 2 1/2" X 1/4" Structural Steel Angles

Weight - 3.81 lbs/lin ft

Length - 6'-6" Min

No of Braces - 2

LINE POSTS

Type - "Studded Tee" or "U"

Weight - 1.28 lbs/lin ft (without Anchor)

Length - 6'-0" Min

Anchor - Securely fastened with bearing surface sufficient to resist movement of post. Weight - 0.57 lb Min

METAL END POSTS AND GATE POSTS

Type - 2 1/2" X 2 1/2" X 1/4" Structural Steel Angles

Weight - 3.81 lbs/lin ft

Length - End, 6'-6" Min.; Panel Gate, 7'-0" Min

No of Braces - 1

BRACES (For Corner, End or Line Brace Posts)

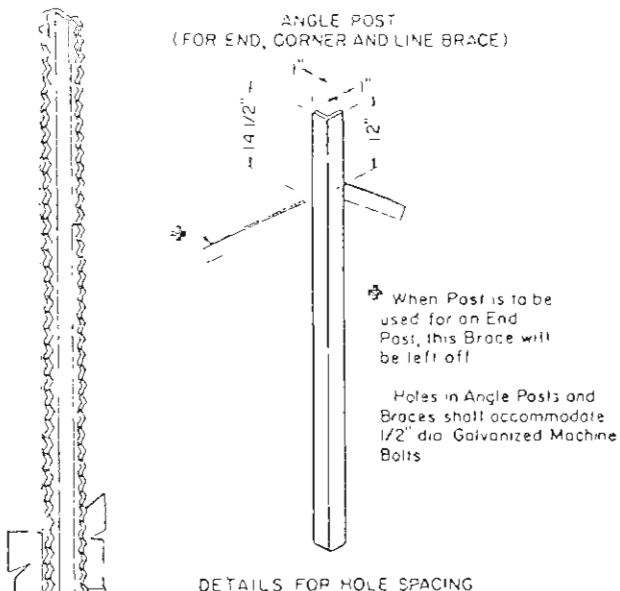
Type - 2" X 2" X 1/4" Structural Steel Angles

Weight - 3.03 lbs/lin ft

Length - Same as corner and end posts used

Posts shall be hot-rolled steel or cold-rolled steel.

TYPICAL METAL POSTS



CONCRETE OR BASE

Concrete shall be Class A

Weight of concrete shall be 150 lbs/cu ft

ALTERNATE WALK GATES

Height - 42"

Weight - 115 lbs (5 panels) complete with latch and hinges

Width of gate opening - 3'-0"

Gate Frame - 3/4" ID Standard Galvanized Pipe or acceptable equivalent and shall be of all welded construction

Mesh shall be of the same construction designated for Driveway Gate.

Alternate equivalent standard metal gates other than shown will be acceptable subject to the Engineer's approval

BARBED WIRE

Steel barbed wire shall conform to ASTM Designation A 121, 12 1/2 Gauge with Class 1 coating, 13 1/2 Gauge with Class 2 Coating or 15 1/2 Gauge with Class 3 Coating.

Aluminum barbed wire shall conform to ASTM Designation B 211, with alloy 5052-0 for the line wire and alloy 5052-H38 for the barbs

WIRE MESH

Wire mesh used in combination wire fence shall be galvanized and conform to the following:

STANDARD	Woven Wire Field For Style or Design No.	ALTERNATE 4" X 4" Wire "V" Mesh
932-6-11 (32" width)	55 lbs/lin ft	34" width - .75 lbs/lin ft
726-6-11 (25" width)	55 lbs/lin ft	25" width - .54 lbs/lin ft
939-6-11 (39" width)	82 lbs/lin ft	Cross wires - 1 strand - 1/2 gauge min
		Horizontal - 2 strand - 12 1/2 gauge

GATES

DRIVEWAY GATES: (PANEL) (ALTERNATE)

Height - approximately 42" (5 panels) --- Width of gate opening - 16'-0" Min.

Weight - Galvanized Steel, 75 lb:

Gates to be of Riveted construction as follows: Min 4 No 10 rivets at each right angle connection and where diagonal braces connect to horizontal panels. Min 3 No 10 rivets where diagonal braces connect to top and bottom panels

DRIVEWAY GATES:

Height - 42"

Weight - Not less than 90 lbs complete with latch and hinges

Width of gate opening - 16'-0"

Gate Frame - 1" ID Standard Galvanized Pipe or acceptable equivalent and shall be of all welded construction

Mesh shall enclose the gate frame as shown and shall be the same mesh design as the fence, or as approved by the Engineer.

WALK GATES

Height - approx 42" (5 panels)

Weight - Galvanized Steel, 10 lbs

Tempered Aluminum, 10 lbs

Width of gate opening - 3'-0"

ALTERNATE WALK GATES

Height - 42"

Weight - 115 lbs (5 panels) complete with latch and hinges

Width of gate opening - 3'-0"

Gate Frame - 3/4" ID Standard Galvanized Pipe or acceptable equivalent and shall be of all welded construction

Mesh shall be of the same construction designated for Driveway Gate.

Alternate equivalent standard metal gates other than shown will be acceptable subject to the Engineer's approval

In lieu of galvanized finish on gate frames, Cadmium Plated pipe or Aluminum painting will be considered to be equivalent.

LATCHES AND HINGES

Galvanized steel or Aluminum of standard make

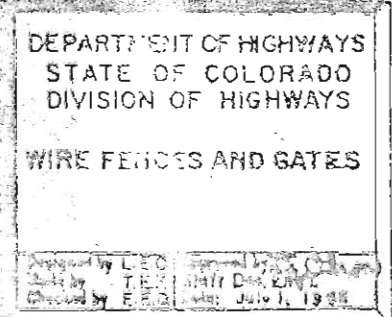
Hinges shall be placed as shown to prevent theft

In lieu of standard make latches it will be permissible to use an electro-galvanized chain, eyebolt and snaphook type latch

Eyebolt, chain and snaphook assembly to be secured to latch side of gate. Gate closure affected by wrapping chain around and post and snaphook hook into chain.

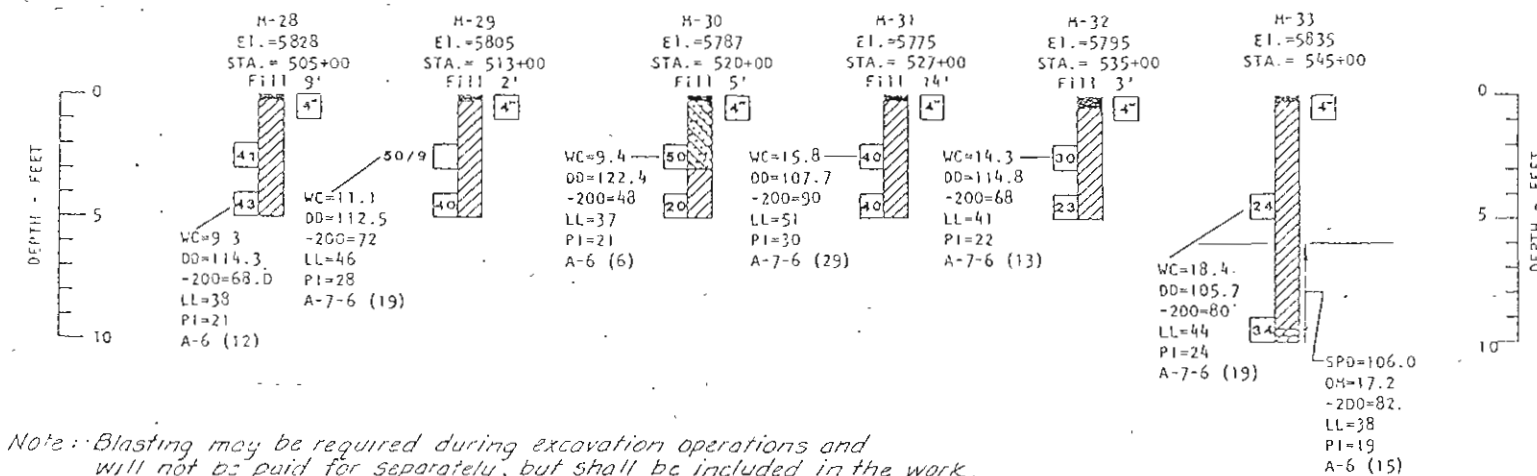
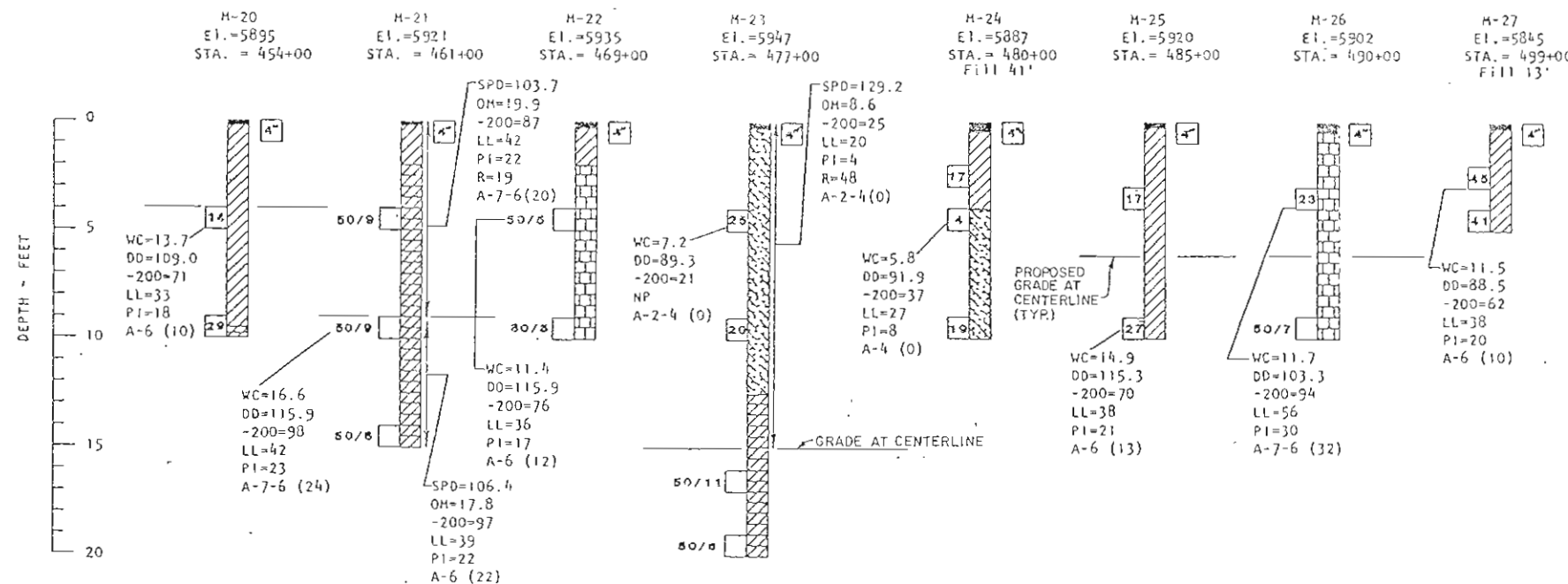
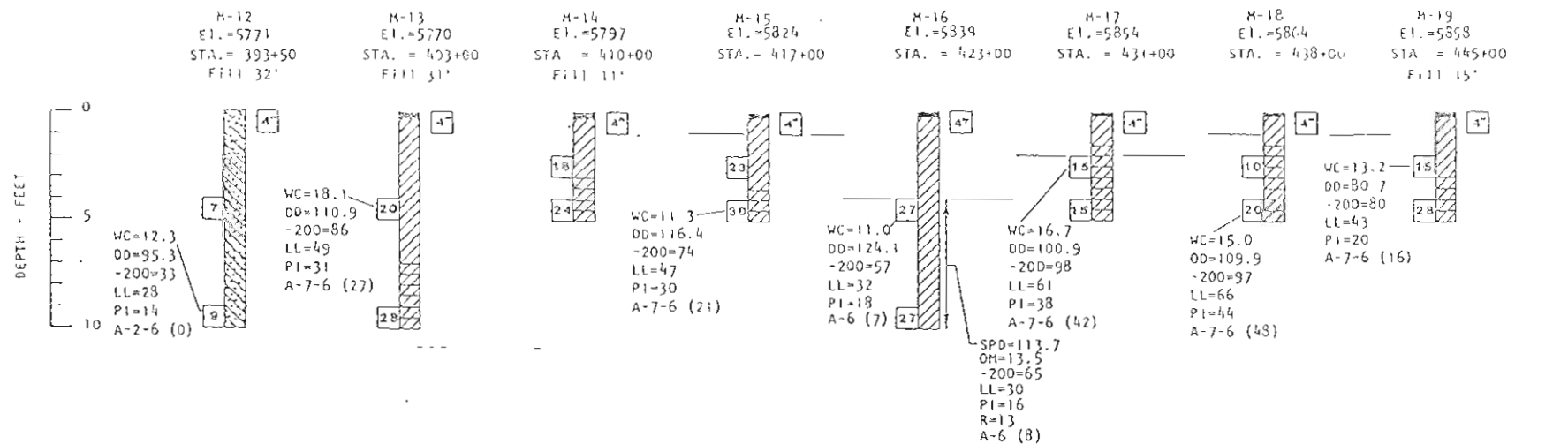
WOODEN STAYS

Wooden stays shall be untreated native timber. Stay dimensions, including length, shall have a tolerance of: ± 1/4"



LOGS OF EXPLORATORY HOLES

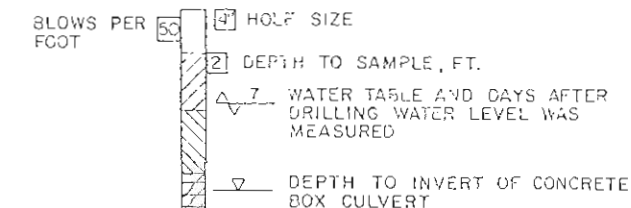
AS CONSTRUCTED	FED. ROAD REGION	DIVISION	PROJ. NO.	SHEET NO.	SHEET TOTALS
NO REVISIONS <u>7</u> / REVISED <u> </u> VOID <u> </u>	VIII	CCLO.	1,747-1(5)	15	49



LEGEND

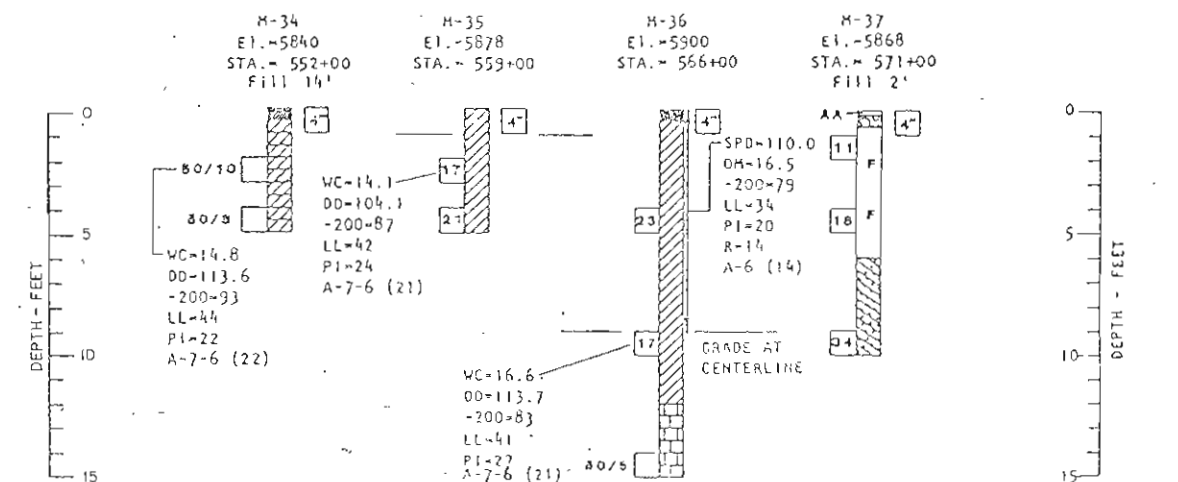
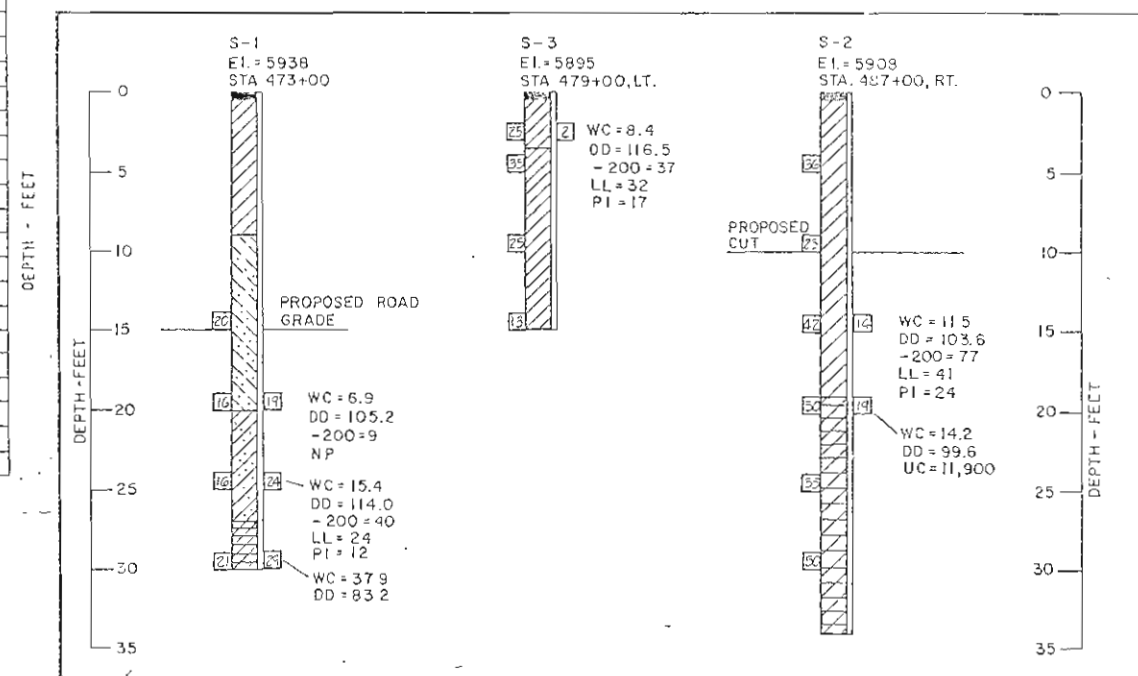
TOPSOIL	LIMESTONE
BASE COURSE	SILTY
FILL	CLAYEY
WATER	AA ASPHALT
SAND	1 3/8 INCH I.D. SPLIT SPOON SAMPLE
CLAY	2 INCH I.D. CALIFORNIA LINER SAMPLE
CLAYSTONE	SHELBY TUBE SAMPLE
SANDSTONE	

TEST BORING:



LABORATORY TEST RESULTS:

WC = WATER CONTENT (%)
 DD = DRY DENSITY (pcf)
 +4 = PERCENTAGE RETAINED ON #4 SIEVE
 -200 = PERCENTAGE PASSING NO 200 SIEVE
 LL = LIQUID LIMIT (%)
 PI = PLASTICITY INDEX (%)
 NP = NON-PLASTIC
 UC = UNCONFINED COMPRESSIVE STRENGTH (psf)
 SPD = STANDARD PROCTOR DENSITY (pcf)
 OM = OPTIMUM MOISTURE CONTENT (%)
 R = R VALUE

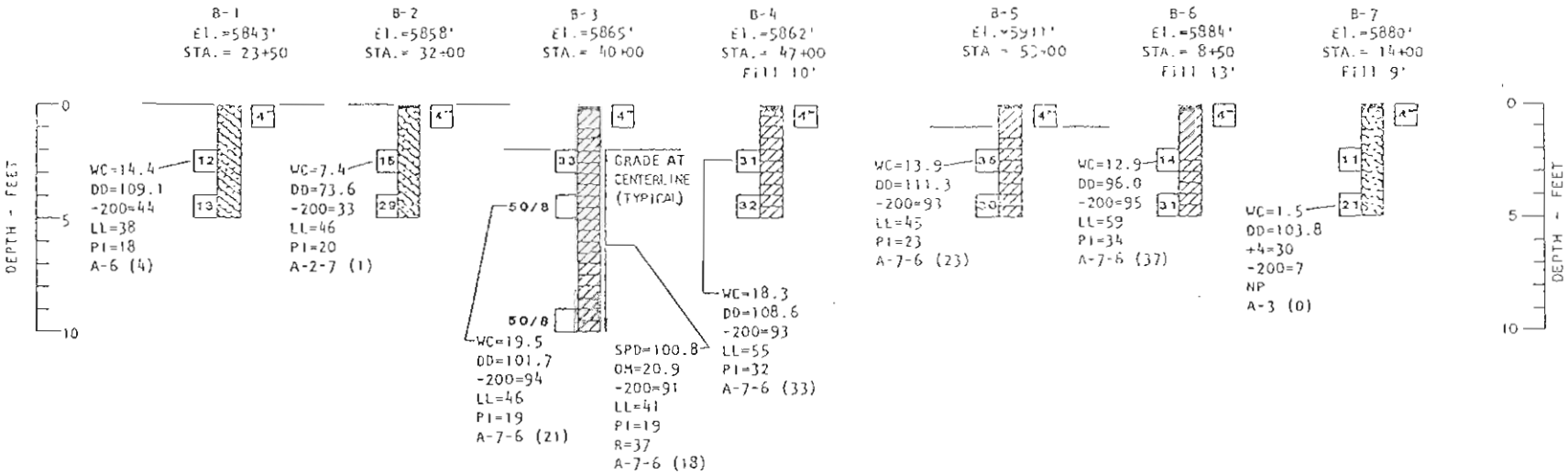


Note: Blasting may be required during excavation operations and will not be paid for separately, but shall be included in the work.

STATE OF COLORADO
DEPARTMENT OF HIGHWAYS
DIVISION OF HIGHWAYS
FORM NO. 11
FEB. 1964

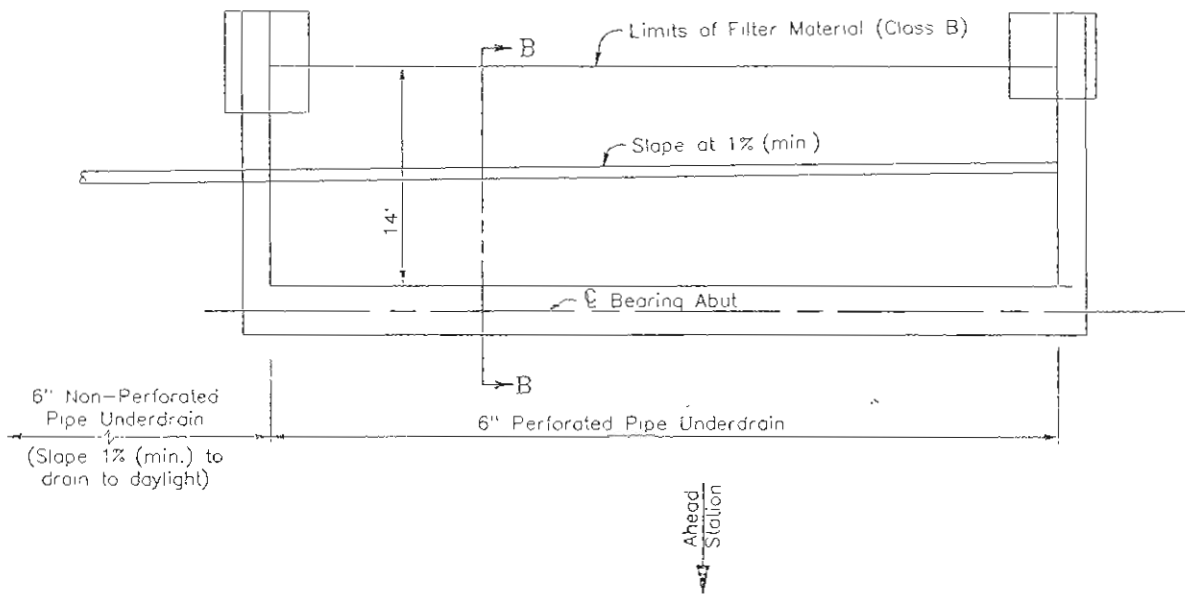
LOGS OF EXPLORATORY HOLES

AS CONSTRUCTED				
NO REVISIONS	7-2-61	REVISED	VOID	
FED. ROAD REGION	DIVISION	PROJ. NO.	SHEET NO.	SHEET TOTALS
VIII	COLO.	IXFU 470-1(55)	16	47

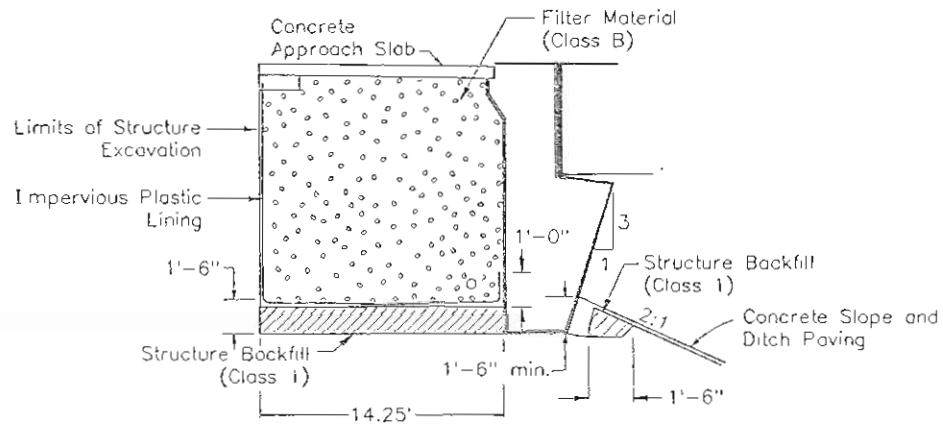


DETAILS OF BACKFILL AT BRIDGE ABUTMENTS
FOR STRUCTURES NO. F-16-NE,

ABUTMENT
PLAN



SECTION B-B



NOTE: Structure Excavation, Structure Backfill and Filter Material (Class B) behind Abutments shall be as indicated in this detail. All other Structure Excavation and Backfill for Retaining Walls, Front of Abutments and Slope and Ditch Paving shall be as shown in Standards M-206-1 and M-206-2.

AS CONSTRUCTED			REVISED	7-27-67	NO. 1	NO. 2	NO. 3	NO. 4	NO. 5	NO. 6	NO. 7	NO. 8	NO. 9	NO. 10	NO. 11	NO. 12	NO. 13	NO. 14	NO. 15	NO. 16	NO. 17	NO. 18	NO. 19	NO. 20	NO. 21	NO. 22	NO. 23	NO. 24	NO. 25	NO. 26	NO. 27	NO. 28	NO. 29	NO. 30	NO. 31	NO. 32	NO. 33	NO. 34	NO. 35	NO. 36	NO. 37	NO. 38	NO. 39	NO. 40	NO. 41	NO. 42	NO. 43	NO. 44	NO. 45	NO. 46	NO. 47	NO. 48	NO. 49	NO. 50	NO. 51	NO. 52	NO. 53	NO. 54	NO. 55	NO. 56	NO. 57	NO. 58	NO. 59	NO. 60	NO. 61	NO. 62	NO. 63	NO. 64	NO. 65	NO. 66	NO. 67	NO. 68	NO. 69	NO. 70	NO. 71	NO. 72	NO. 73	NO. 74	NO. 75	NO. 76	NO. 77	NO. 78	NO. 79	NO. 80	NO. 81	NO. 82	NO. 83	NO. 84	NO. 85	NO. 86	NO. 87	NO. 88	NO. 89	NO. 90	NO. 91	NO. 92	NO. 93	NO. 94	NO. 95	NO. 96	NO. 97	NO. 98	NO. 99	NO. 100
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NOT TO SCALE

GENERAL NOTES

For additional Abutment and Wingwall Details, refer to the Bridge Plans
For Structure Backfill requirements not shown, refer to Standards M-206-1 and M-206-2.
Impervious Plastic Lining shall have a thickness of 30 mils
Quantities for Structure Excavation are included in the Bridge Summary of Quantities

Tabulation of items to be placed behind Abutments for Structures listed below:

PAY ITEM		STR. F-16-NE	STR. F-16-NE	STR. F-16-NE	TOTAL
206	Structure Backfill (Cl 1)	114 cu. yd.			114 cu. yd.
206	STRUCTURE BACKFILL (Cl. 2)	114 cu. yd.			114 cu. yd.
206	Filter Material (Cl B)	1037 cu. yd.			1037 cu. yd.
506	Impervious Plastic Lining	573 sq. yd.			573 sq. yd.
605	6 inch non-perforated pipe underdrain	78 lin. ft.			78 lin. ft.
605	6 inch perforated pipe underdrain	180 lin. ft.			180 lin. ft.

Quantities carried to Structure Quantities sheet.

GENERAL NOTES

ALL WORK SHALL BE DONE ACCORDING TO THE APPLICABLE SECTIONS OF THE STANDARD SPECIFICATIONS OF THE DIVISION OF HIGHWAYS, STATE OF COLORADO.

SEE STANDARD H-208-1 AND H-208-2 FOR STRUCTURE EXCAVATION AND BACKFILL DETAILS NOT SHOWN IN THE ROADWAY PLANS

ROADWAY EMBANKMENT SHALL BE PLACED CONCURRENTLY WITH STRUCTURE BACKFILL (CLASS 1) AND FILTER MATERIAL (CLASS B)

SEE STANDARD H-614-12 FOR STRUCTURE NUMBER INSTALLATION DETAILS.

EXPANSION JOINT MATERIAL SHALL MEET AASHTO SPECIFICATION M-213. PAYMENT FOR EXPANSION JOINT MATERIAL SHALL BE INCLUDED IN THE PRICE BID FOR THE VARIOUS CLASSES OF CONCRETE.

BRIDGE COMPRESSION JOINT SEALER SHALL BE CLOSED CELL, FOAMED, CROSS-LINKED ETHYLENE VINYL ACETATE/LOW DENSITY POLYETHYLENE COPOLYMER WITH UNCOMPRESSED DIMENSIONS AS CALLED FOR ON THE PLANS. BRIDGE COMPRESSION JOINT SEALER SHALL BE INSTALLED WITH AN EPOXY BONDING AGENT AS PER THE MANUFACTURER'S INSTRUCTIONS AND SHALL BE ONE CONTINUOUS PIECE AT EACH JOINT WHEN POSSIBLE.

ALL EXPOSED CONCRETE SURFACES EXCLUDING SIDEWALKS AND MEDIAN DOWN TO 1'-0" BELOW FINISHED GRADE SHALL BE FINISHED WITH A CLASS 1 FINISH FOLLOWED BY A CLASS 5 COLORED COATING MEETING THE REQUIREMENTS OF THE REVISION TO SECTION 601, STRUCTURAL CONCRETE (COATING), IN THE SPECIAL PROVISIONS. THE COLOR OF THE CLASS 5 COATING SHALL BE IN THE RED SERIES NO. 31667 (TAN) OR NO. 30219 OF FEDERAL STANDARD NO. 3959 AS ILLUSTRATED ON DMS 92 HEREIN.

THE LOCATION OF ALL CONSTRUCTION JOINTS SHALL BE APPROVED BY THE ENGINEER.

THE FOLLOWING TABLE GIVES THE MINIMUM CLASS B LAP SPlice LENGTH FOR CONTINUOUS REINFORCING BARS UNLESS OTHERWISE SHOWN.

BAR SIZE	#4	#5	#6	#7	#8	#9	#10	#11
SPlice LENGTH FOR CLASS A CONCRETE	1'-10"	2'-4"	3'-0"	4'-0"	5'-4"	6'-8"	8'-6"	10'-5"
SPlice LENGTH FOR CLASS S CONCR.	1'-10"	2'-4"	2'-9"	3'-3"	3'-11"	4'-11"	6'-3"	7'-8"

STAGGER ALL SPLICES ONE SPlice LENGTH WHERE POSSIBLE

APPLIED WIND AND EARTHQUAKE LOADS WERE NOT CONSIDERED IN ANALYZING THE STRUCTURE FOR STABILITY DURING THE CONSTRUCTION STAGES.

EXISTING UTILITIES AS SHOWN ON THE PLANS WERE PLOTTED FROM THE BEST AVAILABLE INFORMATION. THE CONTRACTOR'S ATTENTION IS DIRECTED TO PARAGRAPH 107.1B OF THE STANDARD SPECIFICATIONS CONCERNING UTILITIES. THE CONTRACTOR SHALL CALL FOR UTILITY LOCATIONS PRIOR TO MAKING EXCAVATIONS.

ABBREVIATIONS

E.F. = EACH FACE
F.F. = FAR FACE
N.F. = NEAR FACE
B = BASE LINE

DESIGN DATA

CURRENT AASHTO DESIGN SPECIFICATIONS.

COLUMNS WERE DESIGNED WITH USD PRINCIPALS AND THE ULTIMATE STRENGTH OF THE SUPERSTRUCTURE WAS CHECKED. ALL OTHER DESIGN ELEMENTS INCLUDING THE SUPERSTRUCTURE WERE SIZED WITH WSD PRINCIPALS.

LIVE LOAD: AASHTO HS-20-44 AND INTERSTATE ALTERNATE
DEAD LOAD: ASSUMES 50 LBS. PER SQ. FT. FOR BITUMINOUS PAVEMENT AND 8 LBS. PER SQ. FT. FOR FORM WORK.

CLASS A CONCRETE:

$f'_c = 3000\text{psi}$, $f'_c = 1200\text{psi}$, $N = 9$

CLASS S CONCRETE:

SEE CAST-IN-PLACE POSTTENSIONED BOX GIRDER DETAILS

REINFORCING STEEL:

GRADE 60, $F_y = 60,000\text{psi}$, $F_s = 24,000\text{psi}$
ALL REINFORCING STEEL SHALL BE EPOXY COATED UNLESS OTHERWISE NOTED.

STRUCTURAL STEEL (INCLUDING PILING):

AASHTO M-183 (ASTM A-36), $F_s = 20,000\text{psi}$

CROSS REFERENCE IDENTIFICATION



SECTION OR DETAIL IDENTIFICATION

AS CONSTRUCTED			
NO REVISIONS		REVISED 7-15-87	VOID

FED. ROAD REGION	DIVISION	PROJ. NO.	SHEET NO.	TOTAL SHEETS
III	COLD	1XEU 470-1 (55)	18	49

REVISIONS		

SUMMARY OF QUANTITIES

ITEM NO.	DESCRIPTION	UNIT	APPROACH SLABS	SUPER-STRUCTURE	RET. WALL ABUT. 1	ABUT. 1	PIER 2	ABUT. 3	RET. WALL ABUT. 3	TOTALS
206	STRUCTURE EXCAVATION	CU.YD.			154	62	488	417	154	1275
206	STRUCTURE BACKFILL (CLASS 1) <i>DELETED BY DGM FORM 105</i>	CU.YD.			121				82	203
206	STRUCTURE BACKFILL (CLASS 2)	CU.YD.			121		330		82	533
502	DRILLING HOLE TO FACILITATE PILE DRIVING	LIN.FT.	<i>DELETED BY CMO #2</i>			240		240		480
502	<i>FACILITATE PILE DRIVING CMO #2</i>	L.S.				0.5		0.5		1.0
502	STEEL PILING (HP 12X74)	LIN.FT.			230	146		382	131	1089
507	CONCRETE SLOPE AND DITCH PAVING (REINFORCED)	CU.YD.				62.0		88.0		150.0
515	WATERPROOFING (MEMBRANE)	SQ.YD.		581						581
518	BRIDGE COMPRESSION JOINT SEALER	LIN.FT.				38		38		76
518	WATERSTOP (6 INCH)	LIN.FT.				17		17		34
601	CONCRETE CLASS A (BRIDGE)	CU.YD.				107.3	307.3	72.4		487.0
601	CONCRETE CLASS A (WALL)	CU.YD.			79.6				81.4	161.0
601	CONCRETE CLASS S (BRIDGE)	CU.YD.	167.2	1707.33 1987.1		76.4		73.3		2304
602	REINFORCING STEEL	LB.			1672		7490		1672	10,834
602	REINFORCING STEEL (EPOXY COATED)	LB.	14,838	351,039 361,539	4176	15,124	77,323	13,258	4315	460,568
607	FENCE CHAIN LINK SPECIAL	LIN.FT.		774 761						774
613	1-1/2" ELECTRICAL CONDUIT	LIN.FT.					74	66		140
613	2" ELECTRICAL CONDUIT (PLASTIC)	LIN.FT.		408 378						408
613	3" ELECTRICAL CONDUIT (PLASTIC)	LIN.FT.		411 378						411
613	LUMINAIRE HIGH PRESSURE SODIUM (WALL TYPE) (16,000 LUMEN)	EACH					2			2
613	LIGHTING SYSTEM	EA.		20						20
618	PRESTRESSING STEEL WIRE OR STRAND	M KFT		7621						7621

- ① INCLUDES 62 CU.YD. AT ABUTMENT 1 AND 86 CU.YD. AT ABUTMENT 3 FOR CONCRETE SLOPE AND DITCH PAVING EXCAVATIONS.
- ② APPROACH SLAB QUANTITY INCLUDES 82.6 CU. YD. AT ABUTMENT 1 AND 84.6 CU.YD. AT ABUTMENT 3 . SUPERSTRUCTURE QUANTITY INCLUDES 106.5 CU.YD. OF BARRIER CONCRETE.
- ③ INCLUDES 7419 LBS. AT EACH ABUTMENT FOR APPROACH SLABS.
- ④ THIS CONTRACT CALLS FOR 582 SQ.YD. OF WATERPROOFING MEMBRANE IN THE MEDIAN FROM STATION 113+56.49 TO STATION 117+07.35. FUTURE CONSTRUCTION WILL REQUIRE 2105 SQ.YD. OF WATERPROOFING MEMBRANE IN THE ROADWAY FROM STATION 113+56.49 TO STATION 117+07.35.

INDEX OF DRAWINGS

DRAWING NO.	TITLE
B1	GENERAL INFORMATION - SUMMARY OF QUANTITIES
B2	GENERAL LAYOUT
B3	ENGINEERING GEOLOGY
B4	CONSTRUCTION LAYOUT
B5	FOUNDATION PLAN
B6	ABUTMENT 1 AND 3
B7	WINGWALL AND ABUTMENT DETAILS
B8	RETAINING WALL DETAILS
B9	PIER 2
B10	PIER SECTIONS
B11	SUPERSTRUCTURE DETAILS
B12	CAST-IN-PLACE POSTTENSIONED BOX GIRDER DETAILS
B13	CAST-IN-PLACE POSTTENSIONED BOX GIRDER DETAILS
B14	CAST-IN-PLACE POSTTENSIONED BOX GIRDER DETAILS
B15	FENCE CHAIN LINK SPECIAL
B16	SLOPE PAVING DETAILS
B17	APPROACH SLAB DETAILS
B18	INTERIOR LIGHTING (CONCRETE BOX GIRDER)
B19	BRIDGE DECK ELEVATIONS
B20	BRIDGE DECK ELEVATIONS
B21	ROADWAY APPROACHES

BRIDGE DESCRIPTION

2 SPANS (151'-0", 141'-0") CONTINUOUS POST-TENSIONED CONCRETE BOX GIRDER BRIDGE.

QUINCY AVENUE OVER S.H. 470, 89'-2" OVERALL, 27'-0" CURB TO CURB EACH DIRECTION SEPARATED BY 16'-0" RAISED MEDIAN, 1'-7" RAIL WITH 5'-6" WALK LEFT SIDE, 1'-7" RAIL WITH 10'-6" WALK RIGHT SIDE, FENCE CHAIN LINK SPECIAL EACH SIDE, 70°-54'-48.4" SKEW ANGLE EACH BENT.

CENTENNIAL ENGINEERING, INC., ARVADA, COLORADO

DIVISION OF HIGHWAYS

QUINCY AVENUE OVER S.H. 470

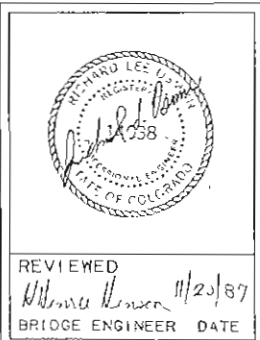
GENERAL INFORMATION
SUMMARY OF QUANTITIES

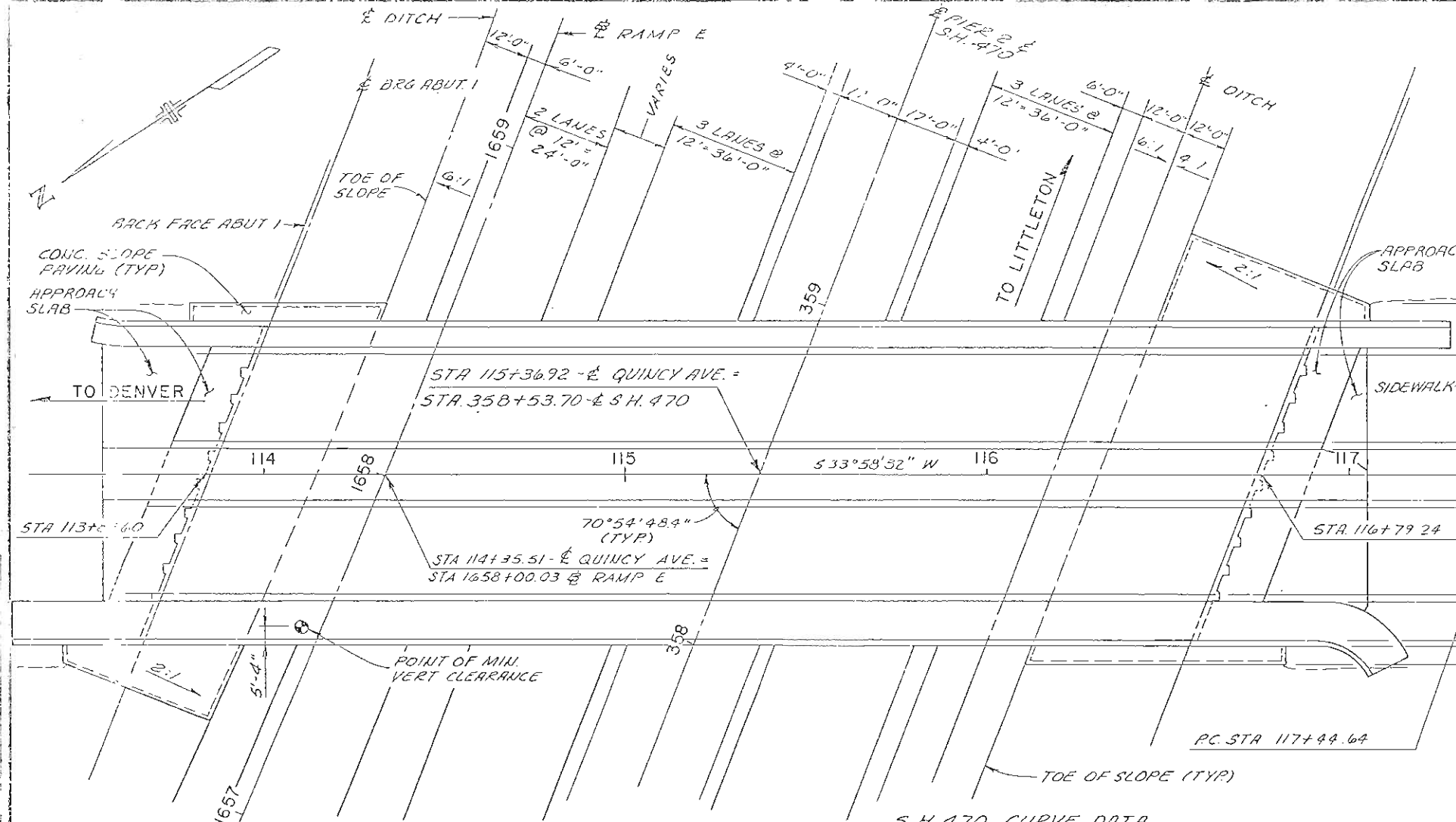
STA. 113+84.60 TO STA. 116+79.24

NEAR MORRISON SEC. 7 T.5S. R.69.W

DESIGNER R. OSMUN	STRUCTURE NUMBER	F-16-NE
DETAILER L. PACHECO		
DRAWING NUMBER B 1	OF 21	DRAWINGS

REVISION DATES (PRELIMINARY STAGE ONLY)





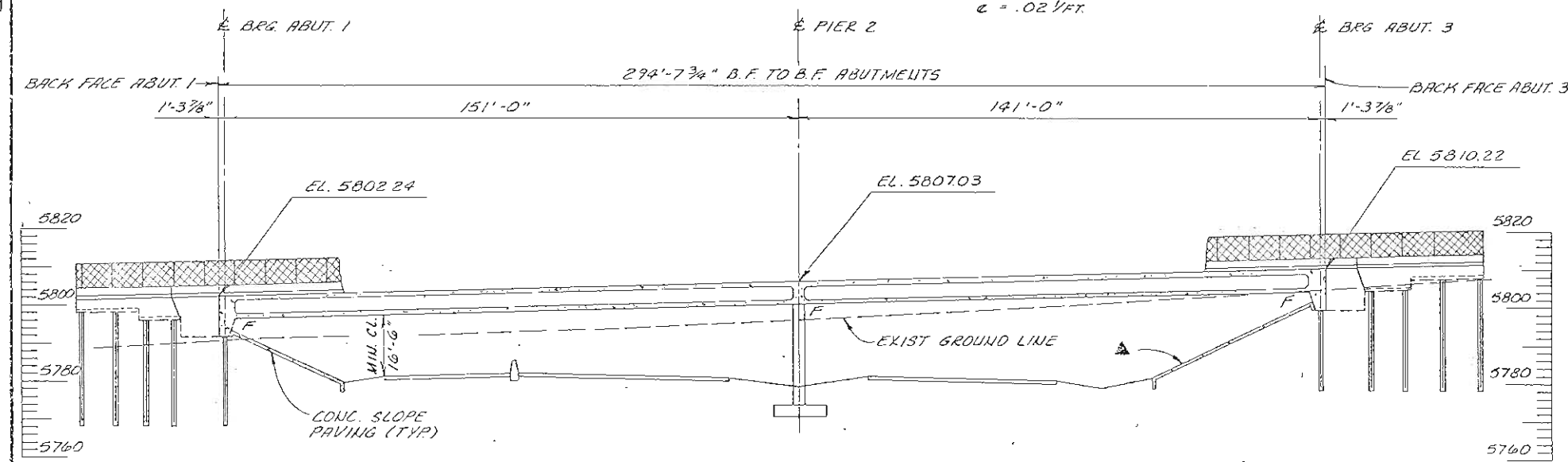
PLAN

S.H. 470 CURVE DATA

PC STA 328+18.68
PI STA 345+86.29
PT STA 363+26.25
 $\Delta = 17^\circ 32' 16''$ RT.
 $D_c = 30.0'$
 $R = 11459.16'$
 $T = 1767.61'$
 $L = 3507.57'$
 $e = .02$ V.F.T.

QUINCY AVE. CURVE DATA

PC STA 117+44.64
PI STA 119+83.12
PT STA 121+63.32
 $\Delta = 68^\circ 32' 17''$ LT.
 $D_c = 16^\circ 22' 13''$
 $R = 350.00'$
 $T = 238.48'$
 $L = 418.68'$
 $e = .037$ V.F.T.



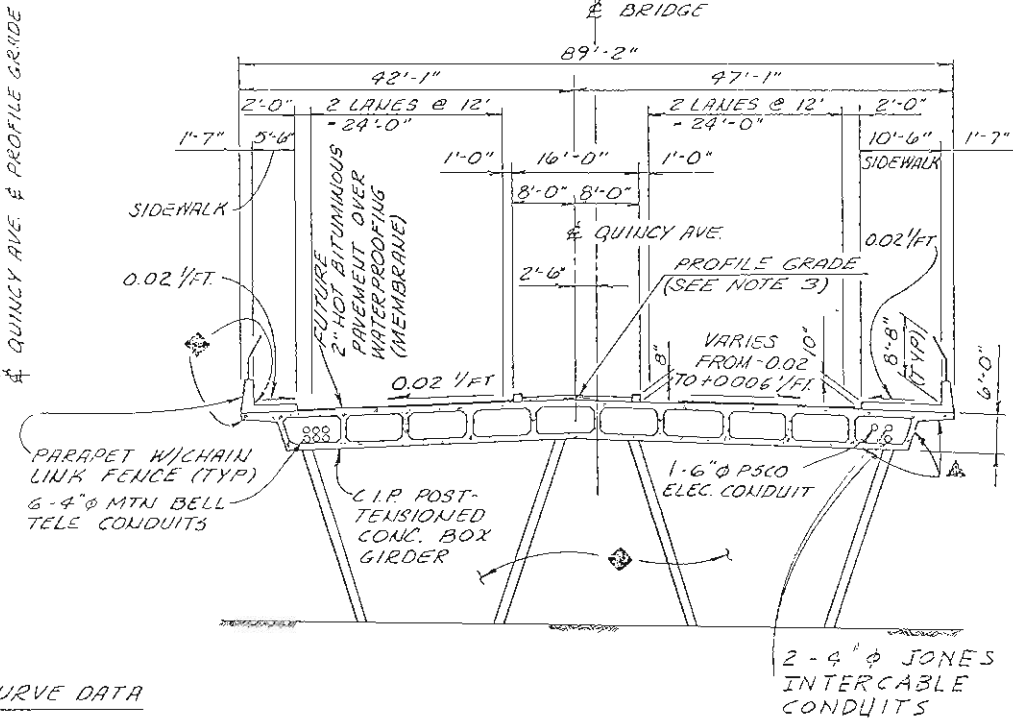
SECTION TAKEN AT Q QUINCY AVE.

- NOTES:
1. LIVE LOADING HS-20-44 & INTERSTATE ALTERNATE.
 2. FINISHED ROADWAY ELEVATIONS ARE AT PROFILE GRADE LINE (Q QUINCY AVE)
 3. THE PROFILE GRADE LINE IS 0.1667' ABOVE THE TOP OF THE CONCRETE DECK.

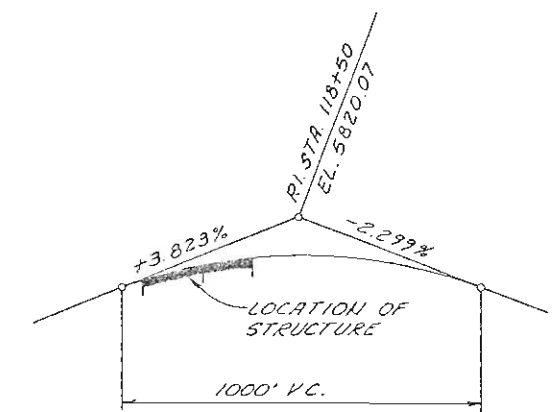
LEGEND:

- ◆ = SURFACE TO RECEIVE COLORED COATING FINISH NO. 31667
▲ = SURFACE TO RECEIVE COLORED COATING FINISH NO. 30219

Q QUINCY AVE & PROFILE GRADE



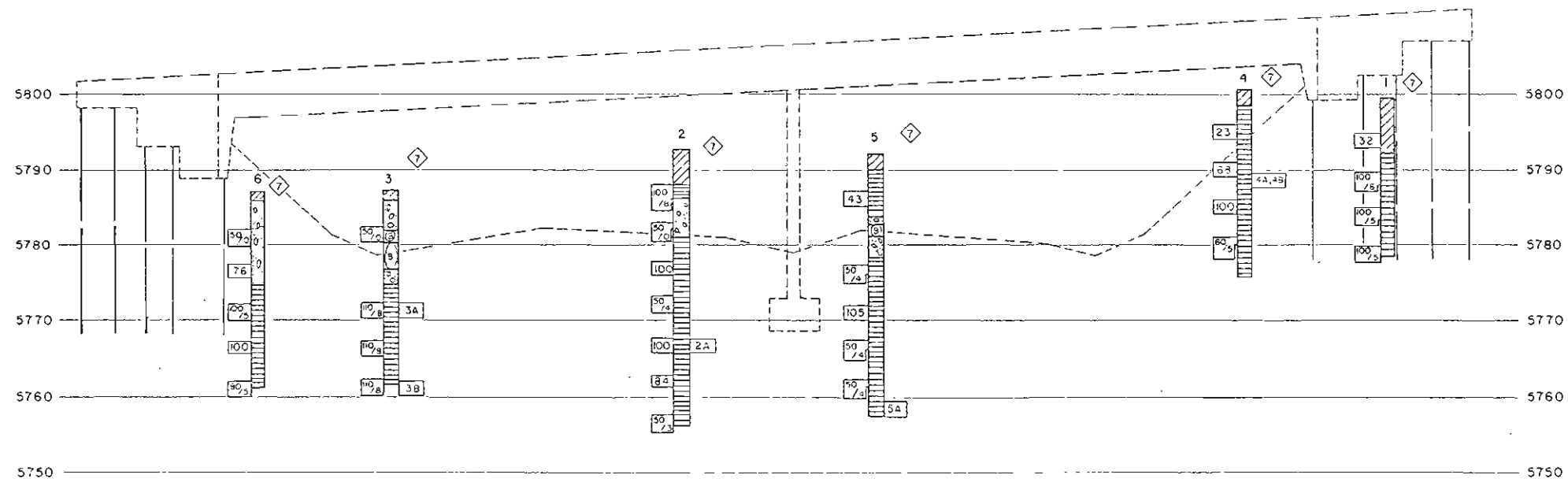
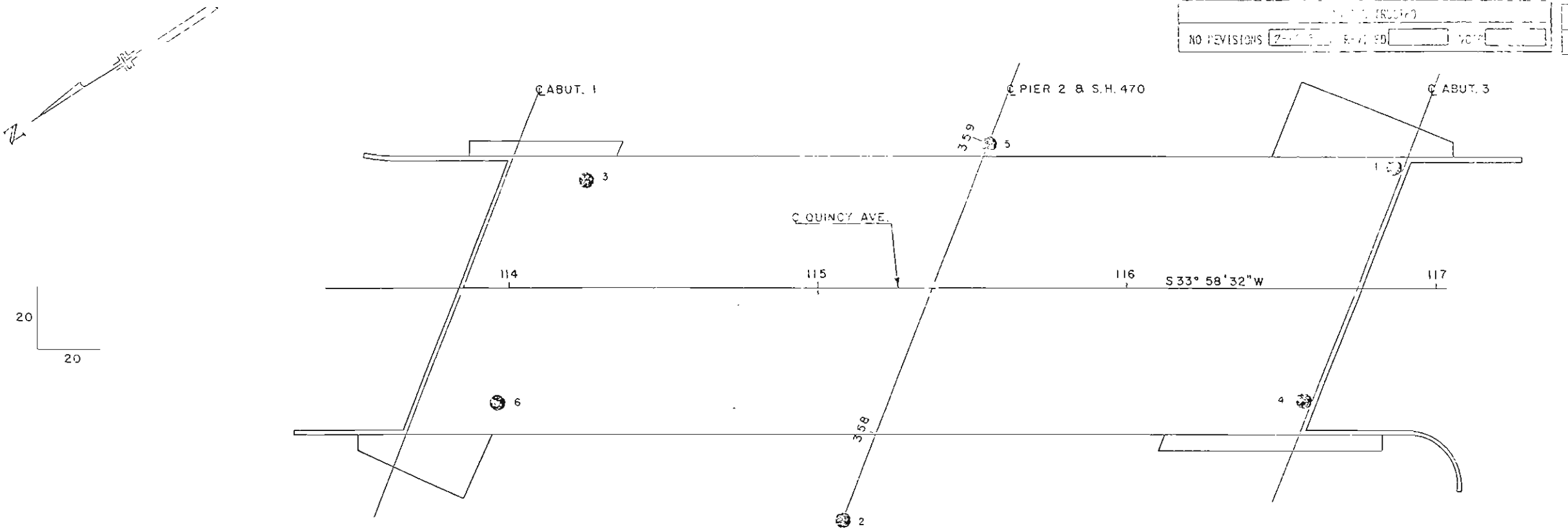
TYPICAL SECTION



PROFILE GRADE

CENTENNIAL ENGINEERING, INC. ARVADA, COLORADO

DIVISION OF HIGHWAYS			
QUINCY AVE. BRIDGE OVER S.H. 470			
GENERAL LAYOUT			
Designer D. OSMUN	Structure F-16-NE		
Detailer L. PACHECO	Numbers		
Drawing Number B2	of 21	Drawings	



SUMMARY OF TEST RESULTS															TYPE OF MATERIAL		LEGEND		DIVISION OF HIGHWAYS		ENGINEERING GEOLOGY		AT C-470	
TEST NO.	DATE	LOCATION	DEPTH (ft)	TEST TYPE	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT	TEST RESULT
2A	25.5-27	CLAYSTONE	A-S(10)	1	2	14	83	14	21	10	20.2													
3A	15.5-16	CLAYSTONE	A-S(14)	0	1	5	54	38	25	13	20.3													
3B	25.5-26	CLAYSTONE		0	0	2	94	39	25	14	21.7													
4A	22.5-23										15.28	120.5												
4B	22.5-23										15.74	130.3												
5A	22.5-23	CLAYSTONE	A-S(18)								147.3	5.65												

CLAY

SANDY GRAVEL

BOULDER

CLAYSTONE

TEST BORING

CONTINUOUS PENETRATION TEST

Location of Test Boring

Location of Continuous Penetration Test

Rotary Boring

Auger Boring

STRUCTURE NO. E-16-N5

NO. 3 3

21

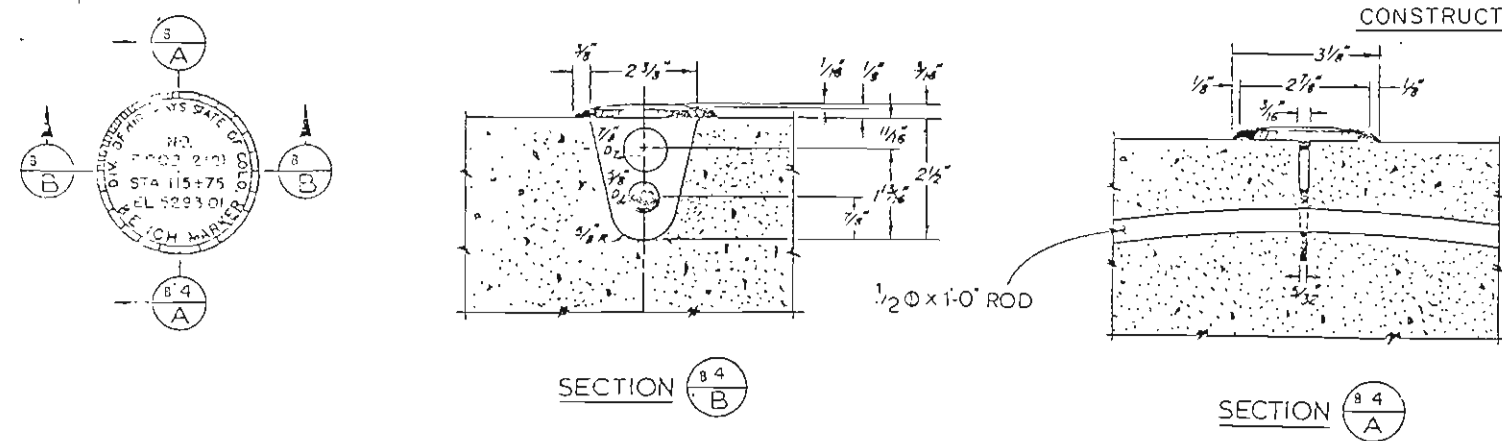
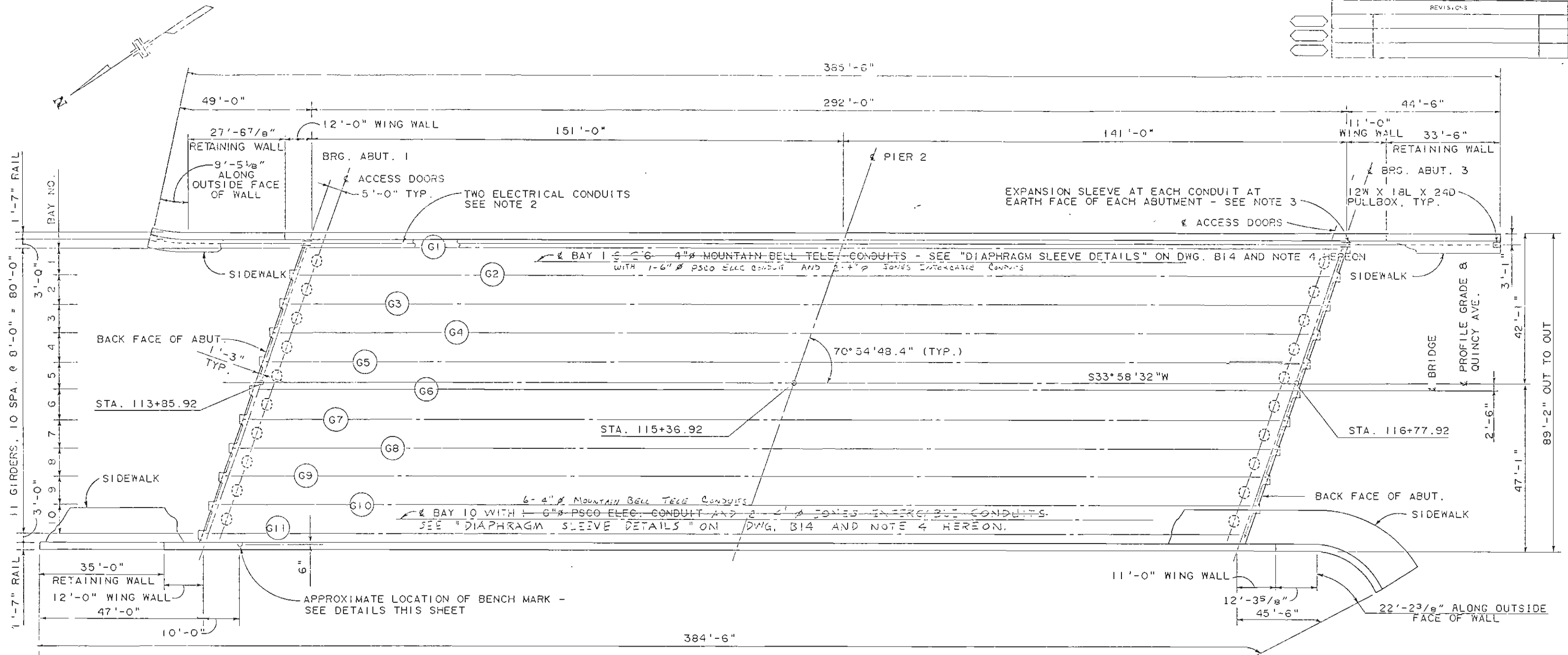
Checked by: [Signature]

Drawn by: [Signature]

Checked by: [Signature]

Date: 11/7/84

		REVISED	



NOTES :

1. ALL EXPOSED SURFACES OF THE BRONZE TABLET ARE TO BE GRIND TO A SMOOTH SURFACE. ALL LETTERS ARE TO BE DEPRESSED A MINIMUM OF $\frac{3}{16}$ ". INFORMATION ON THE BRONZE TABLET INDICATED BY PIN LINES IS TO BE STAMPED IN THE FIELD BY THE ENGINEERING PARTY AFTER THE MARKER HAS BEEN PLACED. $\frac{3}{16}$ " LETTERS AND FIGURES SHALL BE USED.

BRONZE BENCH MARK TABLETS WILL BE FURNISHED BY THE DIVISION AT NO EXPENSE TO THE CONTRACTOR.

INSTALLATION OF THE BRONZE BENCH MARK TABLET WILL NOT BE PAID FOR DIRECTLY, BUT SHALL BE INCLUDED IN THE PRICE BID FOR CONCRETE CLASS 3 (BRIDGE).

THE STATION SHOWN ON THE MARKER SHALL BE THE CENTERLINE STATIONING DIRECTLY OPPOSITE THE MARKER.
2. PLACE ONE 3" ELECTRICAL CONDUIT FOR FUTURE TRAFFIC SIGNALS AND ONE 2" ELECTRICAL CONDUIT FOR FUTURE LIGHTING IN SIDEWALK AS SHOWN ON THE TYPICAL SECTION THRU DECK ON DWG. B11.
3. THE COST OF EXPANSION SLEEVES SHALL BE INCLUDED IN THE PRICE BID FOR ITEM 613. 2" (3") ELECTRICAL CONDUIT.
4. ALL CONDUITS AND MOUNTING HARDWARE WILL BE FURNISHED BY THE INDICATED UTILITIES. THE COST FOR SLEEVES AT ABUTMENT AND PIER DIAPHRAGMS SHALL BE INCLUDED IN THE COST FOR ITEM 601. CONCRETE CLASS 3 (BRIDGE). CONDUITS SHALL EXTEND FROM STATION 113+33.49 TO STATION 117+10.35 AND SHALL BE CAPPED AT EACH END.

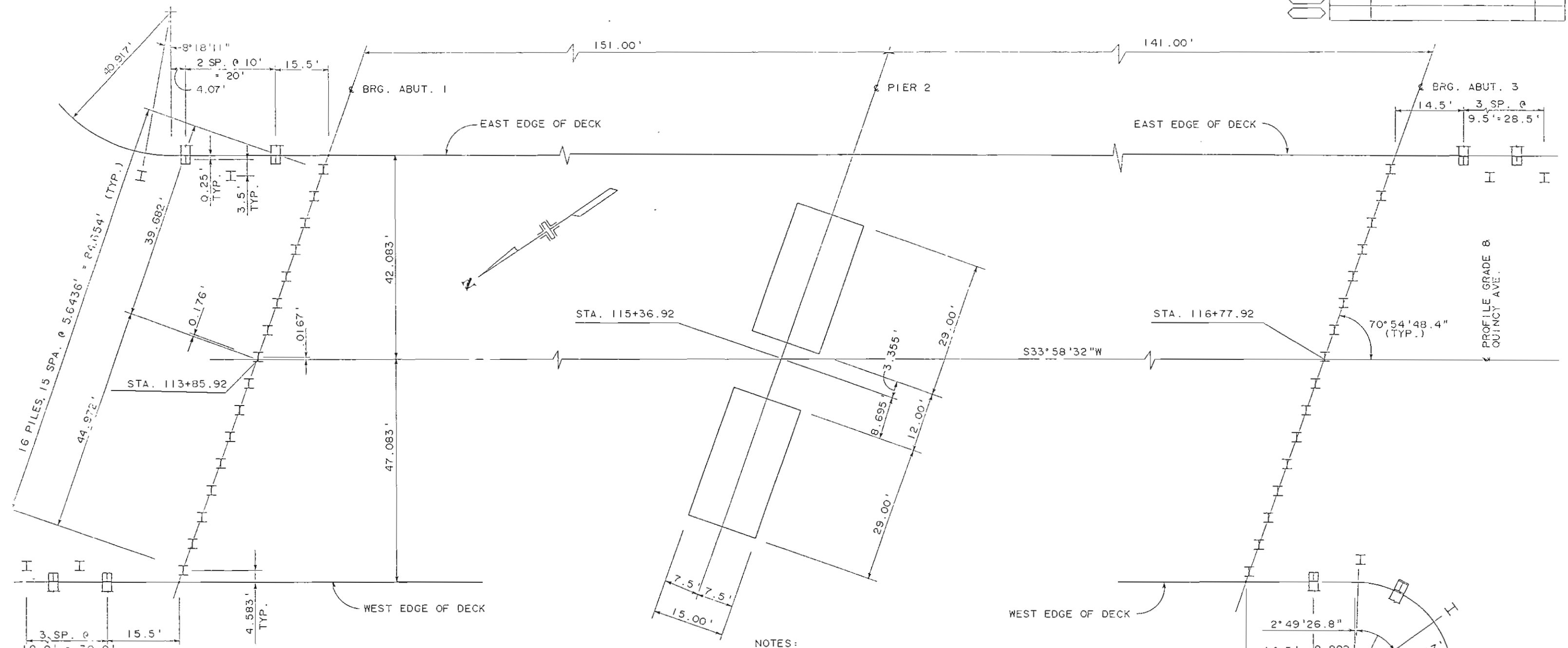
CENTENNIAL ENGINEERING, INC. ARVADA, COLORADO

DIVISION OF HIGHWAYS

CONSTRUCTION LAYOUT

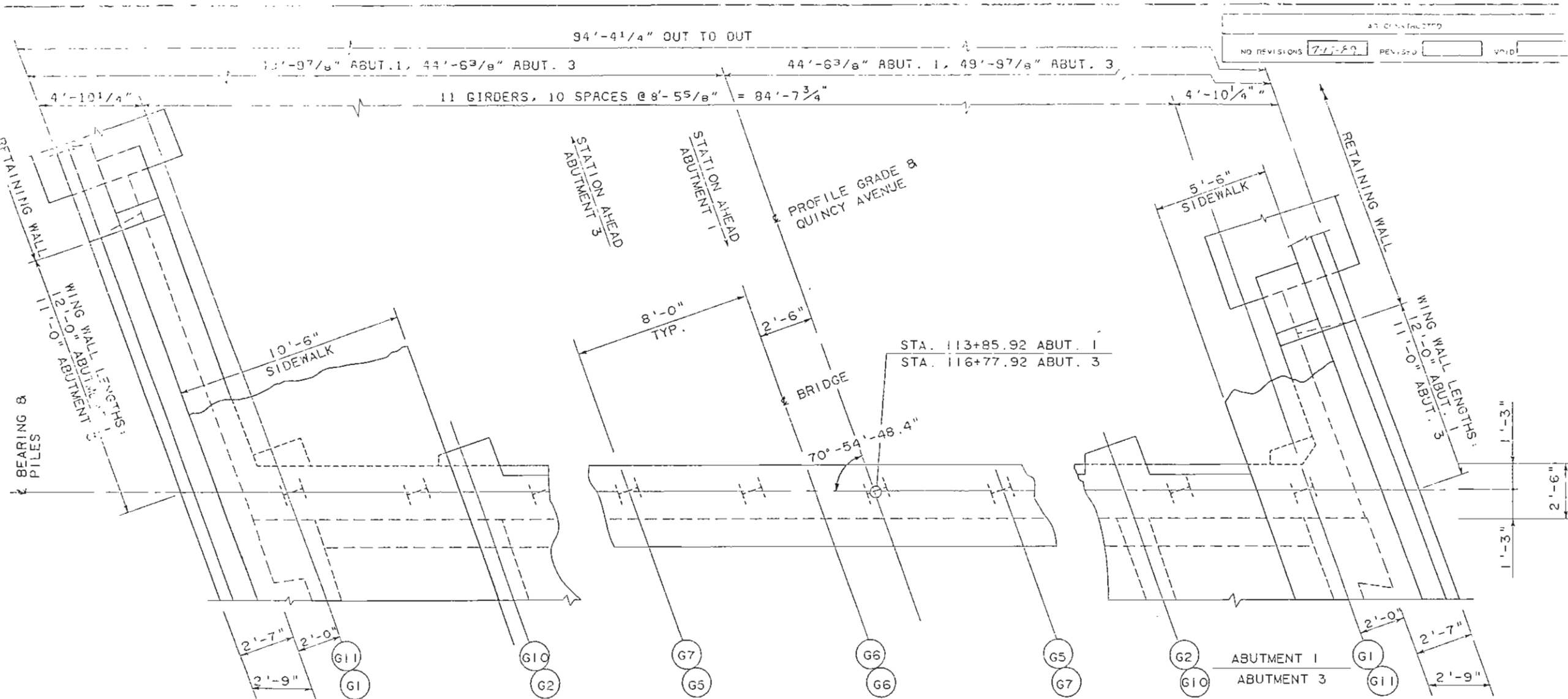
DESIGNER R. OSMON	STRUCTURE	F-16-NE
DETAILER L. PACIFIC	NUMBER	
DRAWING NUMBER B 4 OF 21 DRAWINGS		

AS CONSTRUCTED		FED. ROAD DISTRICT	PROJ. NO.	SHEET NO.	TOTAL SHEETS								
NO. REVISIONS	REVISED 7-15-87	VOID	VIII	COLO.	CU 470-1 (55)								
<table border="1"> <thead> <tr> <th colspan="2">REVISIONS</th> </tr> </thead> <tbody> <tr> <td> </td> <td> </td> </tr> <tr> <td> </td> <td> </td> </tr> <tr> <td> </td> <td> </td> </tr> </tbody> </table>						REVISIONS							
REVISIONS													

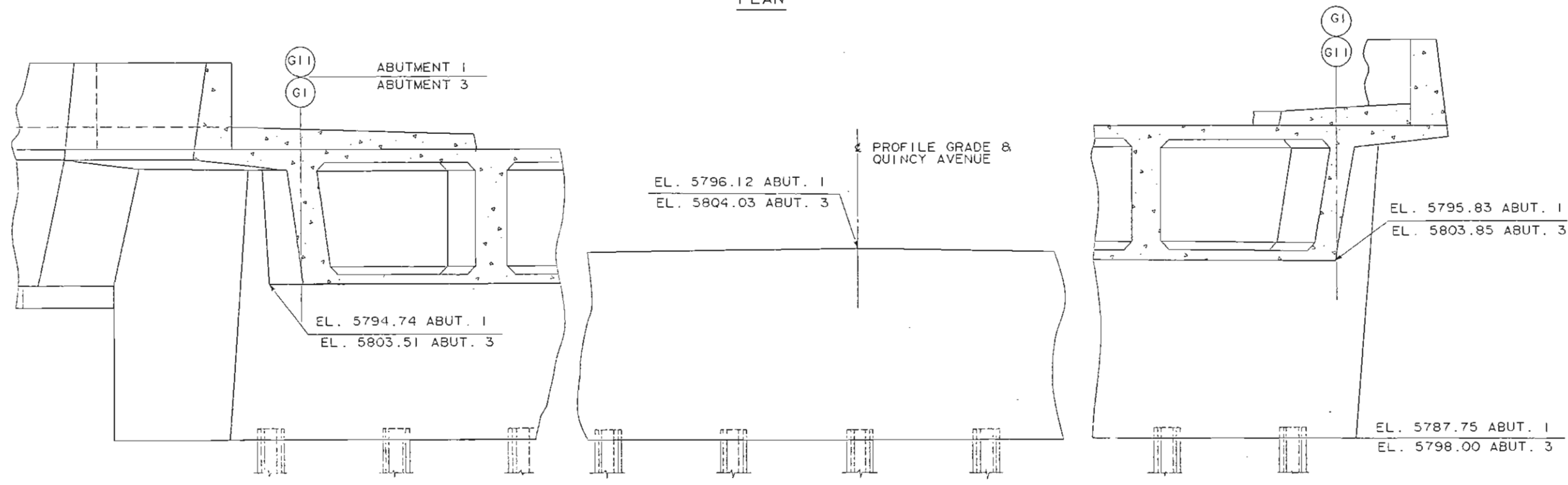


- NOTES:
1. ALL PILES ARE END BEARING.
 2. ALL PILES ARE HP 12 X 74.
 3. MAXIMUM PILE LOAD FOR ABUT. 1 AND ABUT. 3 = 96.8 TONS. FOR GROUP I LOADING.
 4. ESTIMATED PILE TIP ELEVATION IS 5768.0 AT ABUT. 1 AND 5773.0 AT ABUT. 3.
 5. ABUTMENT PILING SHALL BE PLACED IN 18" DIAMETER DRILLED HOLES. HOLES SHALL BE DRILLED TO A DEPTH OF 15' BELOW THE BOTTOM OF THE PILE CAPS AND SHALL BE BACKFILLED WITH A BENTONITE SLURRY BEFORE PILING IS DRIVEN. THE COST OF BENTONITE SLURRY AND THE LABOR TO PLACE IT SHALL BE INCLUDED IN THE WORK.
 6. INDICATES 3:12 BATTERED PILES.
 7. MAXIMUM FOUNDATION PRESSURE FOR PIER 2 = 11.7 KIPS/S.F. FOR GROUP I LOADING.
 8. MAXIMUM PILE LOAD FOR WINGWALL PILES = 32.1 TONS FOR GROUP I LOADING.

CENTENNIAL ENGINEERING, INC. ARVADA, COLORADO	
DIVISION OF HIGHWAYS	
FOUNDATION PLAN	
DESIGNER R. OSMUN	STRUCTURE NUMBER F-16-NE
DETAILER L. PACHECO	
DRAWING NUMBER B 5	OF 21 DRAWINGS



PLAN



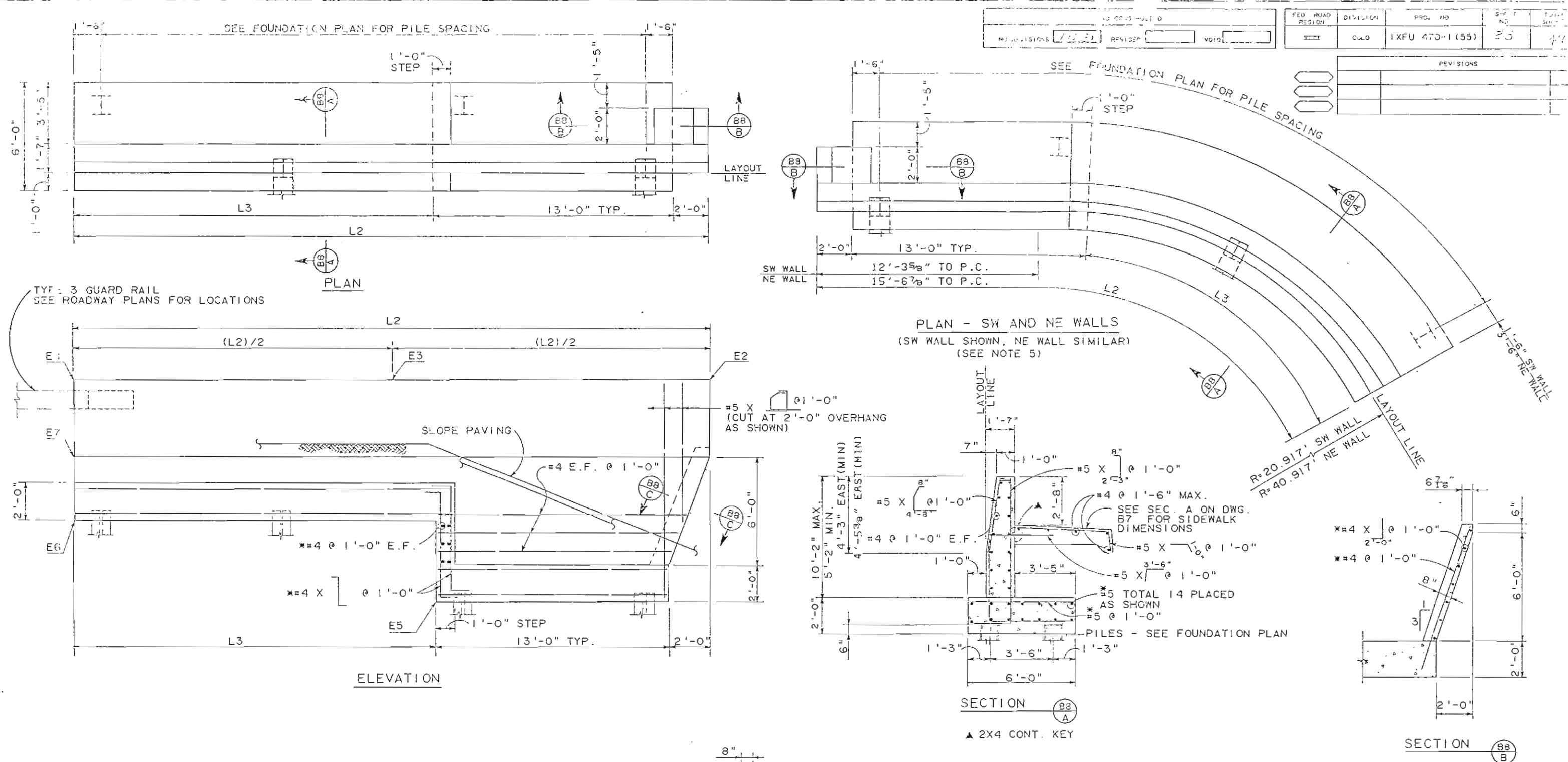
ELEVATION

REV.	AS-BUILT	DIVISION	PROJECT NO.	SHEET NO.	TOTAL SHEETS
1		COLO	IXFU 470-1 (55)	23	44
REVISIONS					

NOTE:

1. APPROACH SLAB NOT SHOWN FOR CLARITY.
2. SEE THE CONSTRUCTION LAYOUT ON Dwg 84 FOR UTILITY LOCATIONS.

CENTENNIAL ENGINEERING, INC. ARVADA, COLORADO		
DIVISION OF HIGHWAYS		
ABUTMENT 1 & 3		
DESIGNER R. OSWALD	STRUCTURE NUMBER	F-16-NE
DETAILER L. PACHECO		
DRAWING NUMBER B 6		OF 21 DRAWINGS
REVISION DATE		PRELIMINARY STAGE ONLY



CENTENNIAL ENGINEERING, INC., ARVADA, COLORADO

DIVISION OF HIGHWAYS

RETAINING WALL DETAILS

DESIGNER R. OSMUN

DETAILER L. PACHECO

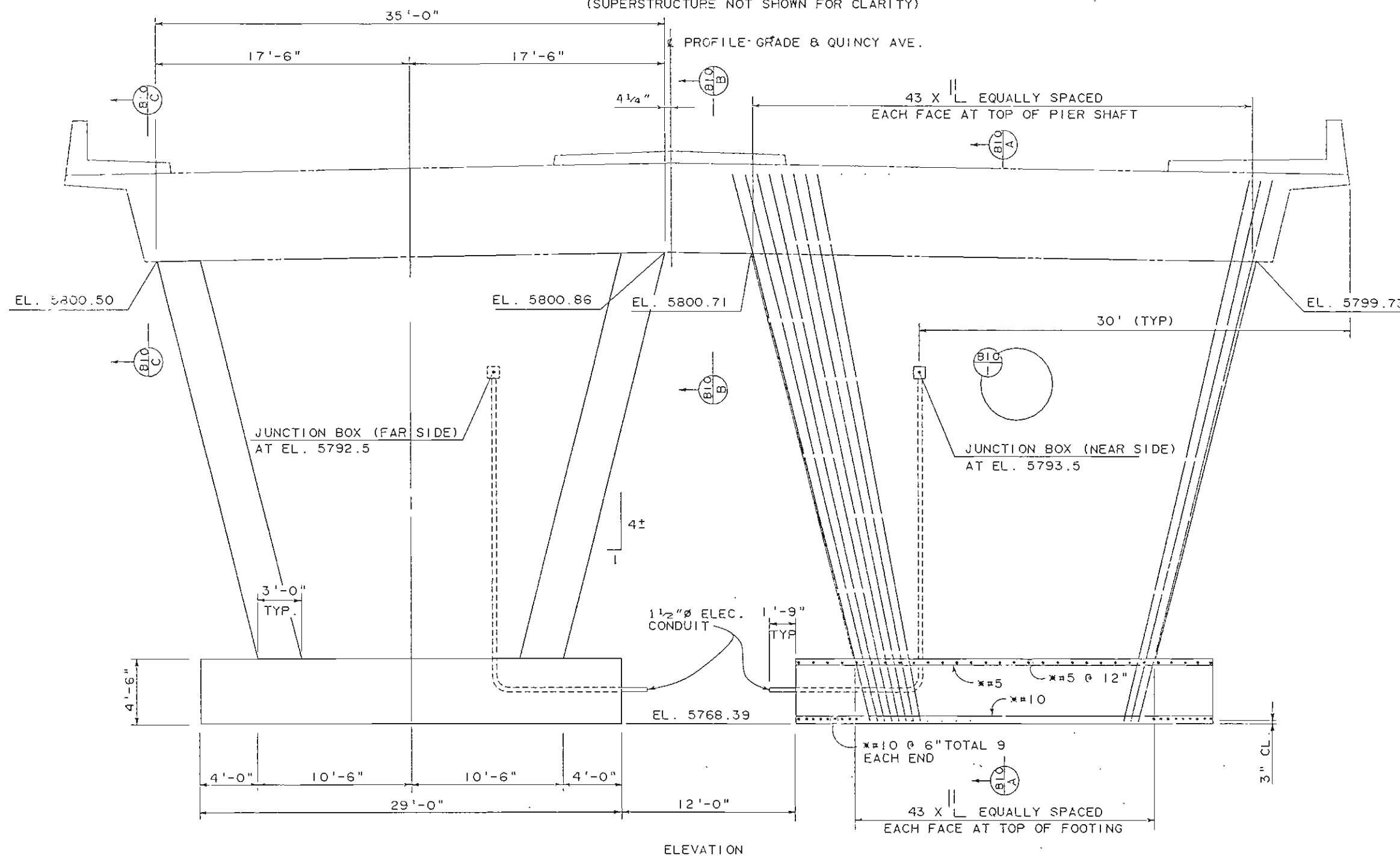
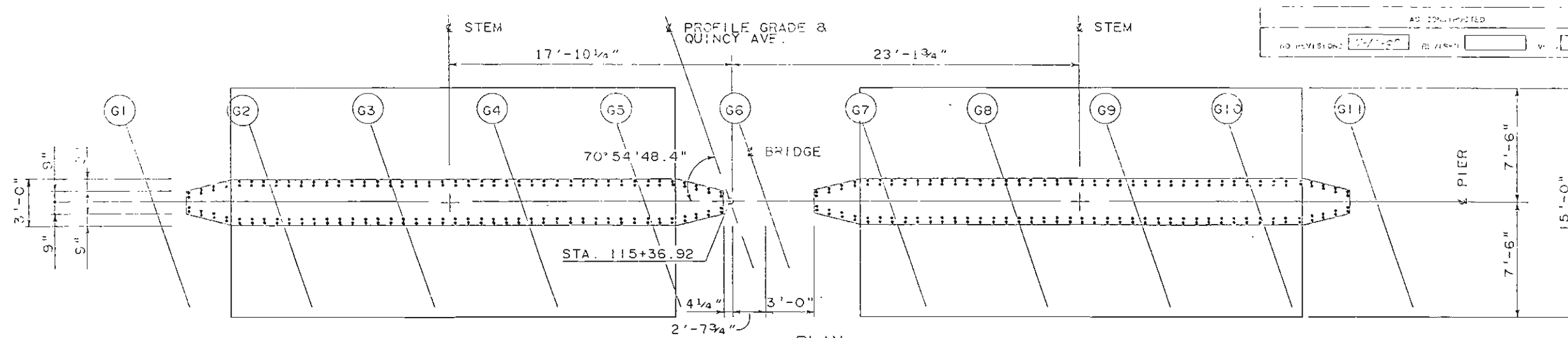
DRAWING NUMBER B 8

OF 21

DRAWINGS

REVISION DATES

USE PRELIMINARY STAGE ONLY



NOTES:

1. USE 2" COVER FOR PRIMARY REINFORCEMENT AND 1 1/2" COVER FOR TIES UNLESS OTHERWISE NOTED.
2. AN * DENOTES UNCOATED REINFORCING STEEL. ALL OTHER REINFORCING STEEL SHOWN ON THIS SHEET SHALL BE EPOXY COATED.
3. DIMENSIONS AND REBAR CALLOUTS ARE TYPICAL FOR BOTH COLUMNS.
4. SEE THE CONSTRUCTION LAYOUT ON DWG. B4 FOR UTILITY LOCATIONS.
5. JUNCTION BOXES SHALL BE FLUSH MOUNTED BOXES WITH WALL MOUNTED LIGHTING FIXTURES.

FED. ROAD REGION	DIVISION	PROJ. NO.	SHEET NO.	TOTAL SHEETS
XIII	COLO.	IXFU 470-1 (55)	26	49

REVISIONS	

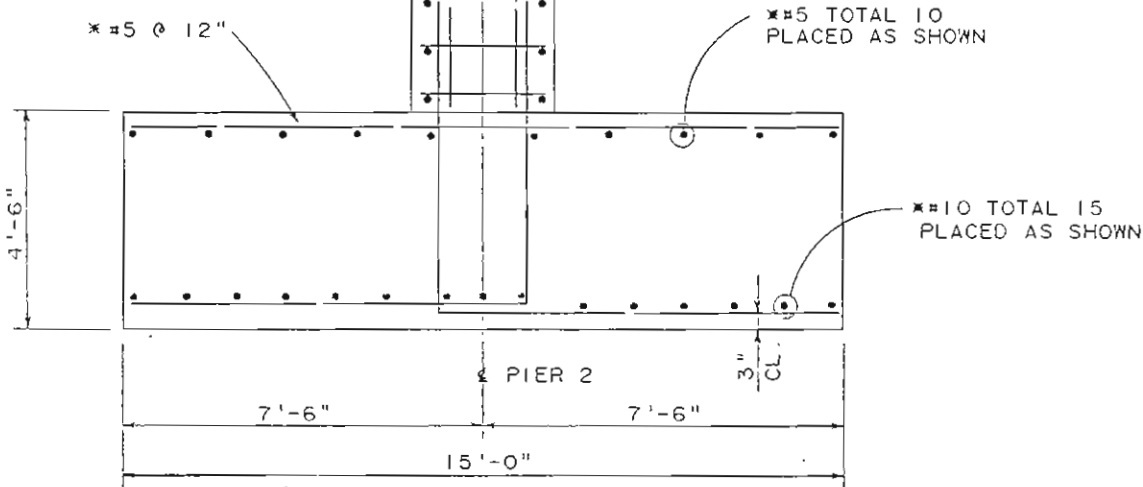
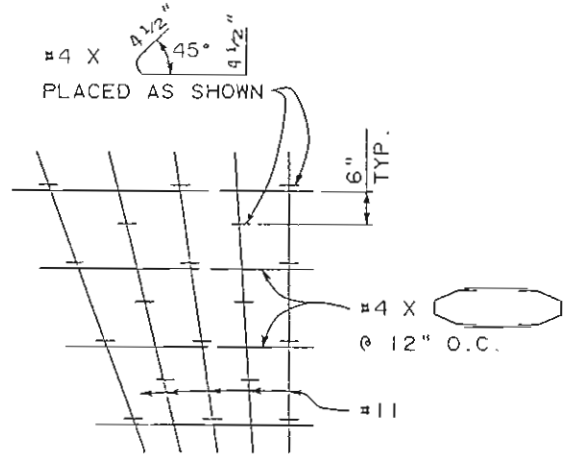
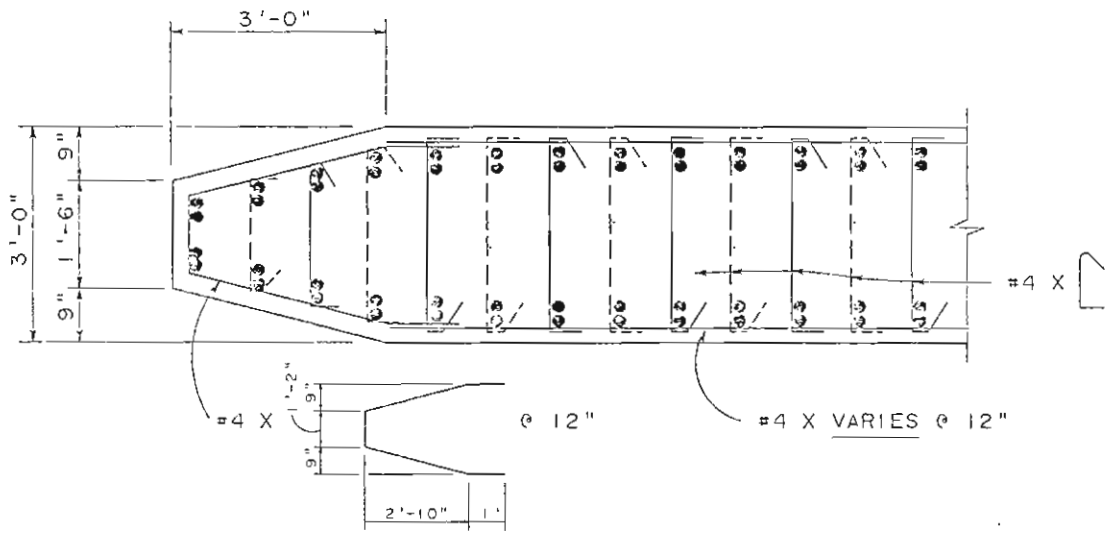
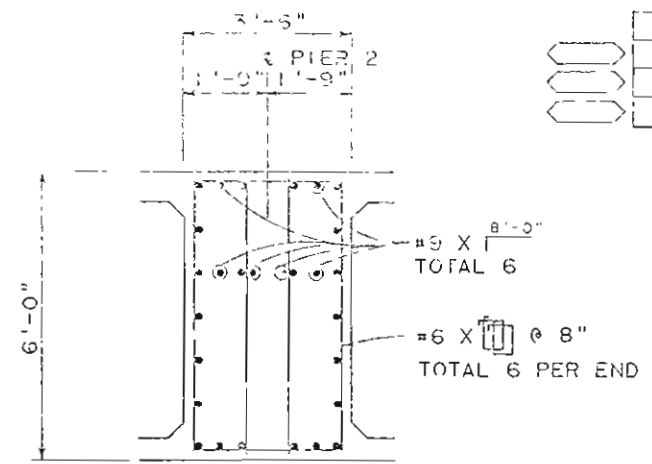
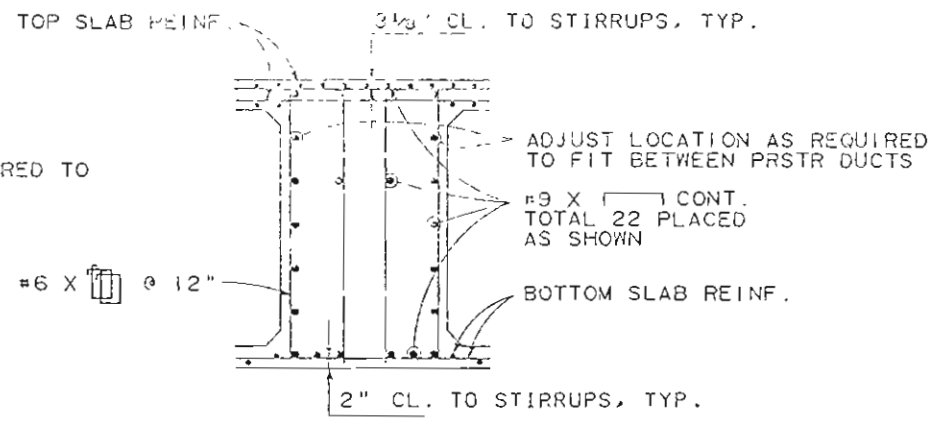
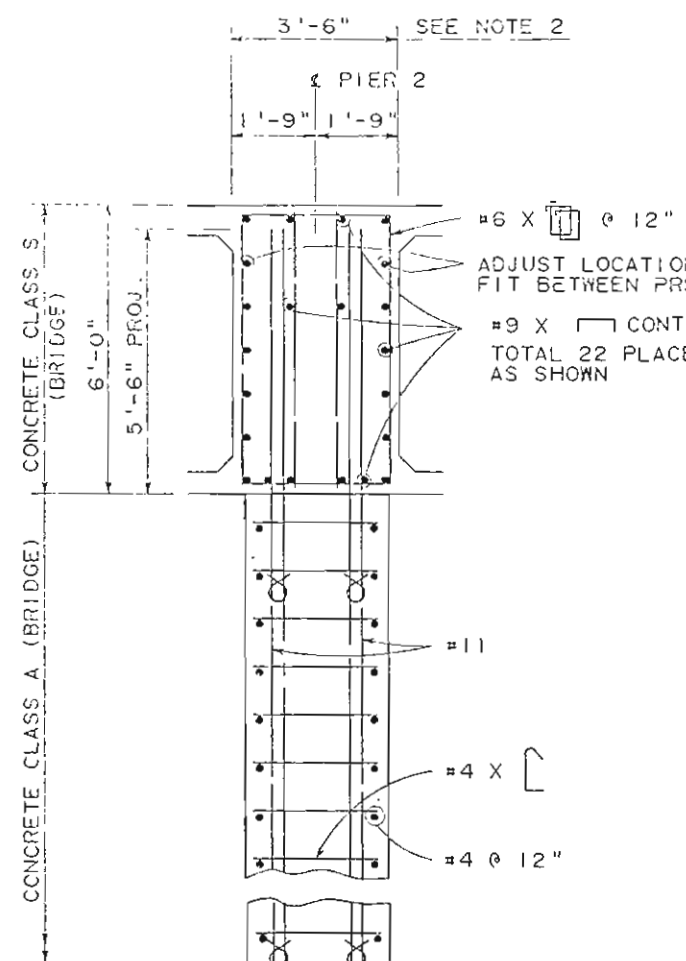
CENTENNIAL ENGINEERING, INC. ARVADA, COLORADO

DIVISION OF HIGHWAYS		
PIER 2		
DESIGNER R. OSWUN	STRUCTURE NUMBER	F-16-NE
DETAILER L. PACHECO	DRAWING NUMBER B 9	OF 21 DRAWINGS

REVISION DATES (PRELIMINARY DRAFT ONLY)

NO. 1	SYMBOL	REVISED	DATE	BY	CHKD	APP'D	DATE	BY	CHKD	APP'D	DATE

PROJECT	CD19	IXFU 470-1 (55)	SHEET	27	TOTAL	41
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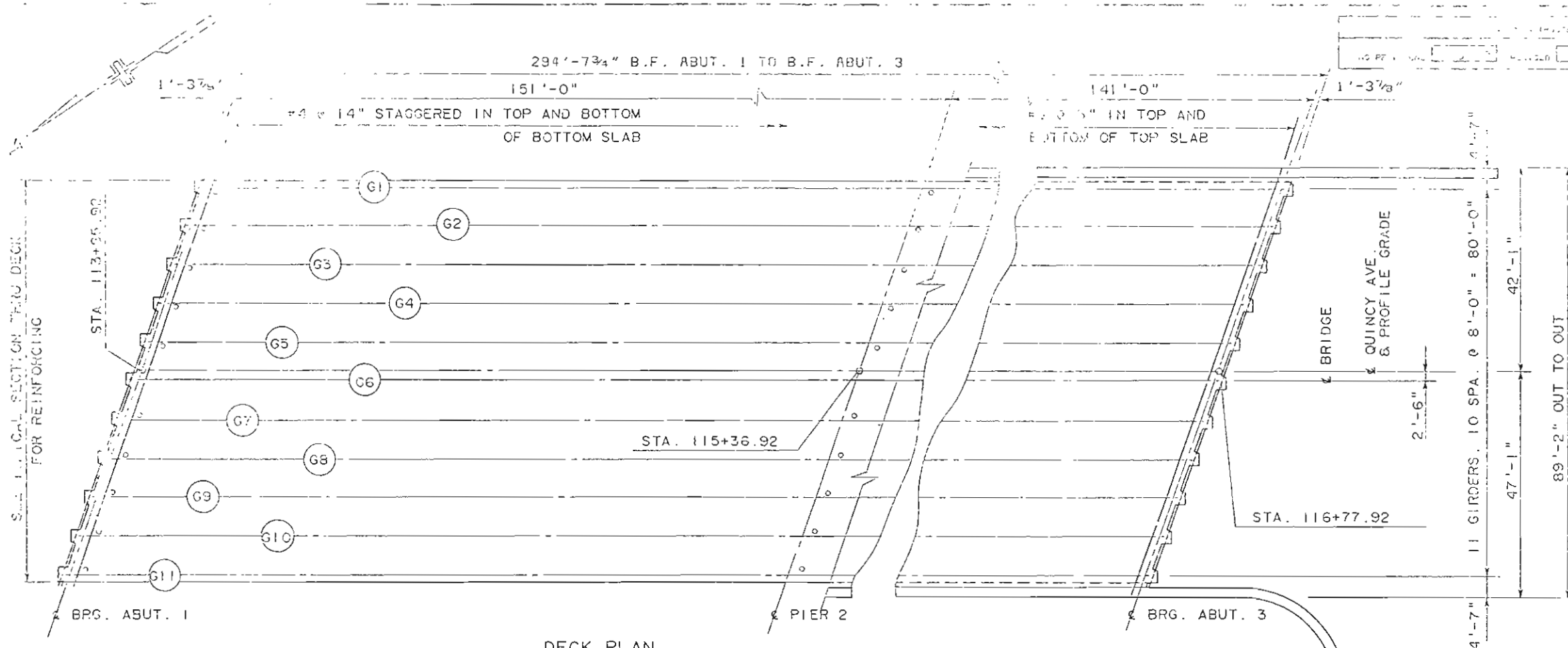
- NOTES:
1. THE CONTRACTOR'S BAR LIST SHALL INCLUDE A DESCRIPTION OF HIS METHODS FOR PLACING AND VIBRATING CONCRETE IN PIER SHAFTS. THE USE OF THE "TIE-AS-YOU-GO" TECHNIQUE, BAFFLES, TREMIES, SHOOTS, PUMPS, ETC. TO PREVENT SEGREGATION OF THE CONCRETE AGGREGATES AND TO ATTAIN THOROUGH VIBRATION IN DIFFICULT PLACES WITHOUT ENTANGLING MECHANICAL VIBRATORS IN THE REINFORCING STEEL SHALL BE ADDRESSED TO THE SATISFACTION OF THE ENGINEER.
 2. DELETE ALL TRANSVERSE DECK REINFORCING IN BOTTOM SLAB AND BOTTOM TRANSVERSE DECK REINFORCING IN TOP SLAB AT PIER CAP.
 3. USE 2" COVER FOR PRIMARY REINFORCEMENT AND 1 1/2" COVER FOR TIES UNLESS OTHERWISE NOTED.
 4. AN * DENOTES UNCOATED REINFORCING STEEL. ALL OTHER REINFORCING STEEL SHOWN ON THIS SHEET SHALL BE EPOXY COATED.

CENTENNIAL ENGINEERING, INC. APPLIED, COLORADO

DIVISION OF HIGHWAYS

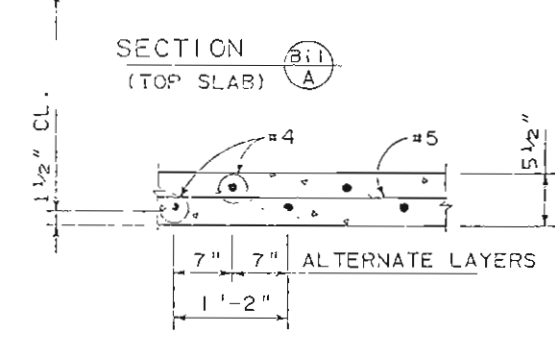
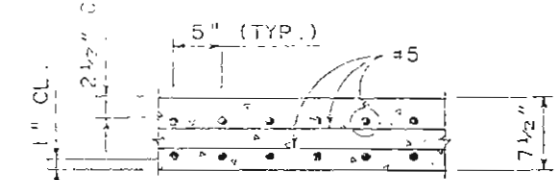
PIER SECTIONS

DESIGNER R. OSBUN	STRUCTURE NUMBER	F-16-NE
DETAILER L. PACHECO	DRAWING NUMBER	B-10
SHEET		21
DRAWING		PIER SECTIONS

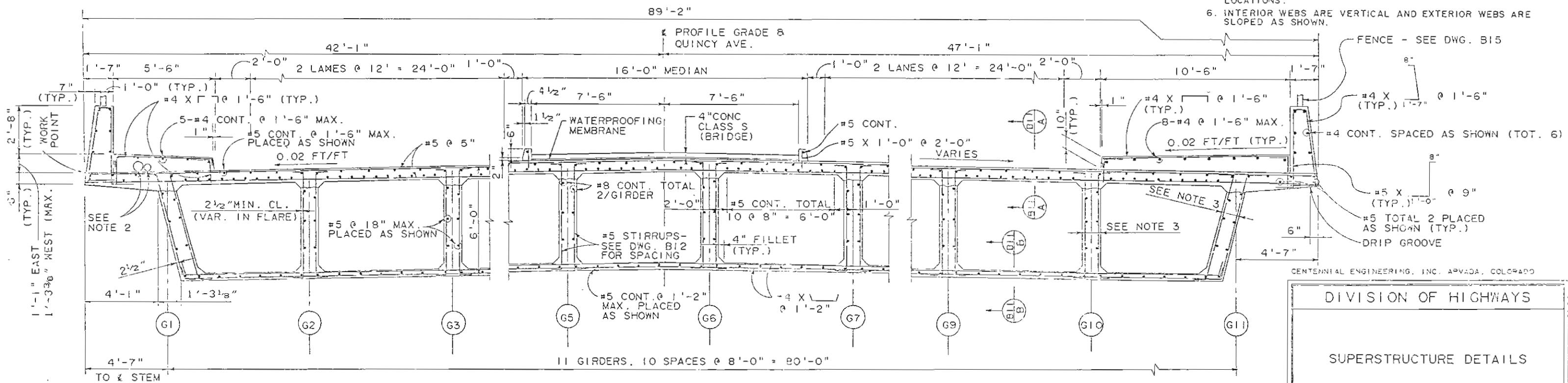


DECK PLAN
(SEE NOTE 4)

REVISIONS				
NO.	DATE	BY	CHKD.	APP'D.
1				
2				
3				
4				
5				
6				
7				
8				
9				
10				

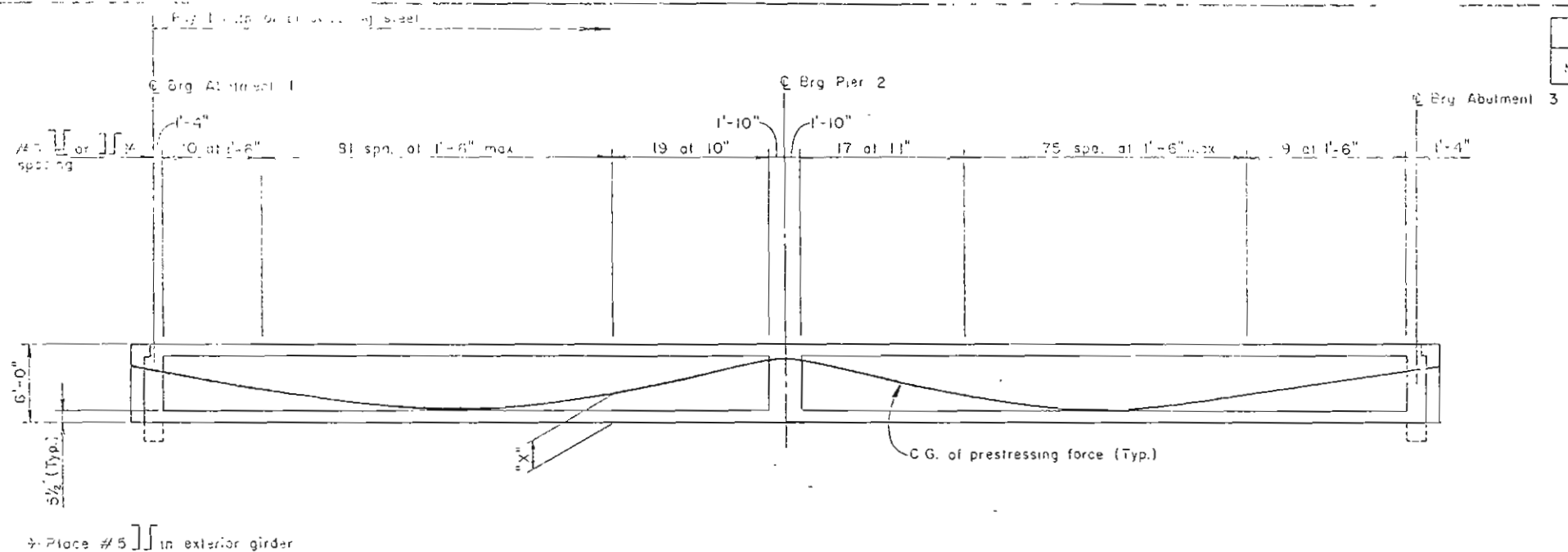


- LEGEND
- BOTTOM SLAB DRAIN - SEE DWG. B.3
- NOTES
1. USE 2" COVER FOR PRIMARY REINFORCEMENT AND 1 1/2" COVER FOR TIES UNLESS OTHERWISE NOTED.
 2. ELECTRICAL CONDUITS, SEE THE CONSTRUCTION LAYOUT ON DWG. B4 FOR SIZE, LOCATION & PURPOSE.
 3. SEE TYPICAL FLARE AT ABUTMENTS AND PIERS ON DWG. B14 FOR DIMENSIONS.
 4. APPROACH SLABS AND ACCESS DOORS NOT SHOWN FOR CLARITY
 5. SEE THE CONSTRUCTION LAYOUT ON DWG. B4 FOR UTILITY LOCATIONS.
 6. INTERIOR WEBS ARE VERTICAL AND EXTERIOR WEBS ARE SLOPED AS SHOWN.



TYPICAL SECTION THRU DECK

CENTENNIAL ENGINEERING, INC. ARVADA, COLORADO	
DIVISION OF HIGHWAYS	
SUPERSTRUCTURE DETAILS	
DESIGNER R. OSWUN	STRUCTURE NUMBER F-16-NE
DETAILER L. PACHECO	
DRAWING NUMBER B 11	OF 21 DRAWINGS



AS CORRECTED		PROJECT NO.	SHEET NO.	TOTAL SHEETS
NO REVISIONS	7-17-21	PERMITS	1XFD 470 (55)	29
REVISIONS				

NOTES

REINFORCING THAT INTERFERES WITH THE PRESTRESSING TENDON ALIGNMENT SHALL BE ADJUSTED AS APPROVED BY THE ENGINEER.

WHERE DEAD END ANCHORAGE AND TENDONS ARE ACCESSIBLE, THE ANCHORAGE SYSTEM AND LENGTH OF PROJECTING PRESTRESSING STEEL SHALL PERMIT JACKING WITH THE SAME JACKING EQUIPMENT THAT WAS USED ON THE LIVE END.

DEVIATIONS FROM THE DUCT PATTERN, DUCT SIZE, AND STRAND SIZE ASSUMED IN THE DESIGN MUST BE APPROVED BY THE ENGINEER. DUCT PATTERNS 1 AND 2 ARE THE ONLY ACCEPTABLE PATTERNS.

THE DEFLECTION SHOWN IS POSITIVE DOWNWARD. IT INCLUDES THE INSTANTANEOUS EFFECTS OF DEAD LOAD AND PRESTRESSING, AND A FACTOR OF THREE (3) MULTIPLIER TO ACCOUNT FOR LONG TERM CREEP. FORMED WEB ELEVATIONS MUST BE ADJUSTED UPWARD FOR AN INDICATED POSITIVE DEFLECTION.

THE DESIGN IS BASED UPON THE USE OF LOW-RELAXATION STRANDS MEETING THE REQUIREMENTS OF ASTM A416 GRADE 270 STRAND. THE CONTRACTOR MAY USE STRESS RELIEVED STRAND MEETING THE REQUIREMENTS OF ASTM A416 GRADE 270 STRAND IF HE ACCOUNTS FOR THE INCREASED STRESS LOSSES RESULTING FROM THE USE OF STRESS RELIEVED STRAND.

STRESSING SEQUENCE

THE DESIGN IS BASED UPON JACKING FROM BOTH ENDS, EITHER SIMULTANEOUSLY OR SEQUENTIALLY. AS AN ALTERNATE, HALF OF THE TENDONS MAY BE JACKED FROM EACH END. IF HALF OF THE TENDONS ARE JACKED FROM EACH END, THE JACKING FORCE SHALL BE INCREASED 300 KIIPS. PRESTRESSING QUANTITIES WILL NOT BE ADJUSTED FOR THIS REQUIRED INCREASE IN THE JACKING FORCE.

NO MORE THAN ONE-HALF (1/2) OF THE PRESTRESSING FORCE IN ANY WEB MAY BE APPLIED BEFORE AN EQUAL FORCE IS APPLIED IN THE ADJACENT WEBS. AT NO TIME DURING THE STRESSING OPERATIONS SHALL MORE THAN ONE-SIXTH (1/6) OF THE TOTAL PRESTRESSING FORCE BE APPLIED ECCENTRICALLY ABOUT THE CENTERLINE OF THE STRUCTURE.

AT THE CONTRACTOR'S OPTION, THE PRESTRESSING FORCE MAY VARY $\pm 5\%$ FROM THE THEORETICAL FORCE PER WEB PROVIDED THE TOTAL P(JACK) FORCE IS OBTAINED AND IS DISTRIBUTED SYMMETRICALLY ABOUT THE CENTERLINE OF THE TYPICAL SECTION. P(JACK) IS THE SUM OF THE PEAK FORCES REACHED DURING JACKING IN EACH TENDON.

DESIGN

THE DESIGN IS BASED UPON THE USE OF LOW-RELAXATION STRAND, A JACKING STRESS OF 0.75 f_s , a 5/8" ANCHOR SET, $K=0.0002$ AND $\mu=0.25$. P(JACK) SPECIFIED AT THE JACKING ENDS INCLUDES FRICTION LOSSES AND PROVIDES FOR AN ADDITIONAL LOSS OF 33 KSI DUE TO ELASTIC SHORTENING AND LONG TERM CREEP AND SHRINKAGE.

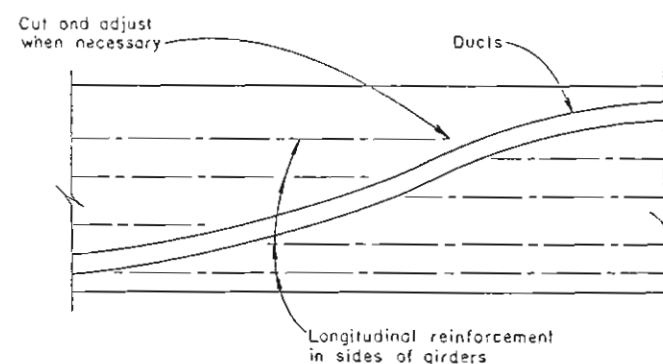
DUCT PATTERN 1, ON DWG. B13, WITH 0.6 INCH DIAMETER STRESS RELIEVED STRANDS IN A 31" O.D. DUCTS WAS ASSUMED IN THE DESIGN.

P(JACK)
 A_s MINIMUM = 25,100 KIIPS TOTAL AT JACKING ENDS
 = 128.9 SQ. IN.
 f_s = 270 KSI
 f_c = 5600 PSI AT 28 DAYS FIELD COMPRESSIVE STRENGTH
 f_{ci} = 4300 PSI AT STRESSING

⊕ DESIGNATES CRITICAL POINTS FOR P(JACK). THE CONTRACTOR SHALL SUBMIT ELONGATION AND JACKING CALCULATIONS BASED ON $KL + \mu$ (INCLUDING ANCHOR SET IF ANY) AND INITIAL STRESS (INITIAL STRESS RATIO TIMES JACKING STRESS BEFORE LONG TERM LOSSES) AT THE POINTS LABELED ⊕ IN THE TABLES HEREON.

Point	1.00	1.10	1.20	1.30	1.40	1.50	1.60	1.70	1.80	1.90	2.00	2.10	2.20	2.30	2.40	2.50	2.60	2.70	2.80	2.90	3.00
"X"	3'-3 1/8"	2'-5 3/8"	1'-7 3/8"	1'-2 1/2"	1'-0 3/8"	1'-2 3/8"	1'-6 3/4"	2'-2 1/4"	3'-0 3/4"	4'-2 1/8"	4'-9 3/8"	4'-3 3/8"	3'-4 1/4"	2'-7 1/2"	2'-1 3/8"	1'-9 3/8"	1'-0 3/4"	1'-4 3/8"	2'-1 1/8"	2'-6 3/8"	3'-3 1/8"
Deflection	0.00'	0.057'	0.111'	0.141'	0.156'	0.153'	0.132'	0.093'	0.054'	0.013'	0.00'	0.015'	0.048'	0.087'	0.125'	0.164'	0.150'	0.135'	0.102'	0.057'	0.00'

WEB ELEVATION



ADJUSTED GIRDER REINFORCING ELEVATION

CRITICAL POINT	$KL + \mu$	INITIAL STRESS RATIO $\times e^{-(KL + \mu)}$
1.0	0.0000	1.0000
1.4	0.0302	0.9703
1.9	0.0659	0.9362
⊕ Δ 1.9677	0.0817	0.9214
2.0	0.0756	0.9244
2.1	0.0575	0.9441
2.6	0.0251	0.9752
3.0	0.0000	1.0000

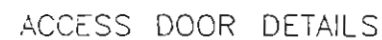
Δ Point of no movement

CENTENNIAL ENGINEERING, INC. ARVADA, COLORADO

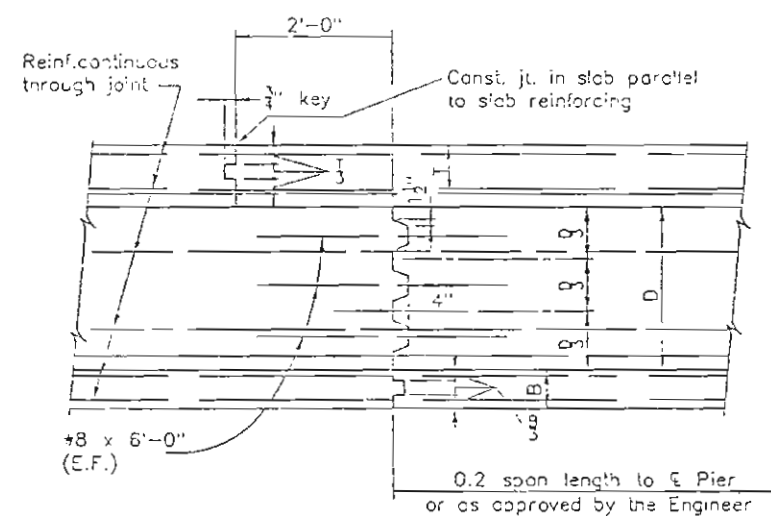
DIVISION OF HIGHWAYS

CAST-IN-PLACE POSTTENSIONED BOX GIRDER DETAILS

Designer: R. OSIMUKI
 Detailer: S. HANSEN
 Drawing Number: B12
 Structure: F-16-NE
 of 21 Drawings

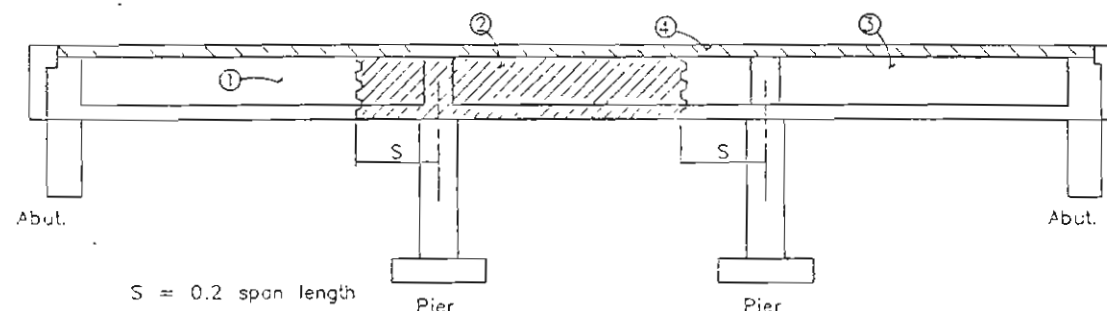


Door, frame, and hardware to be paid for
in Item No. 601 - Concrete Class S.
(Galvanize after fabrication)



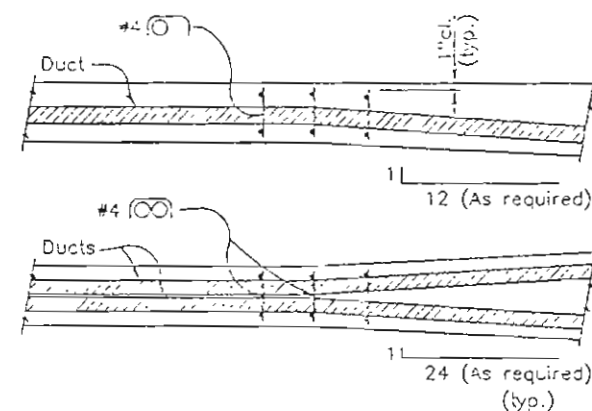
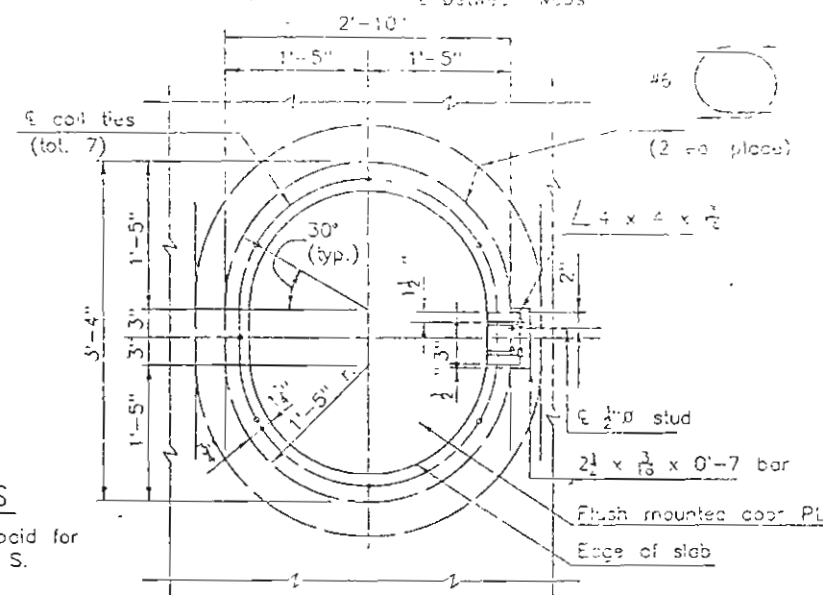
TRANSVERSE WEB CONSTRUCTION JOINT

T = Top slab thickness
B = Bottom slab thickness
D = Top of bottom slab to bottom of fillet



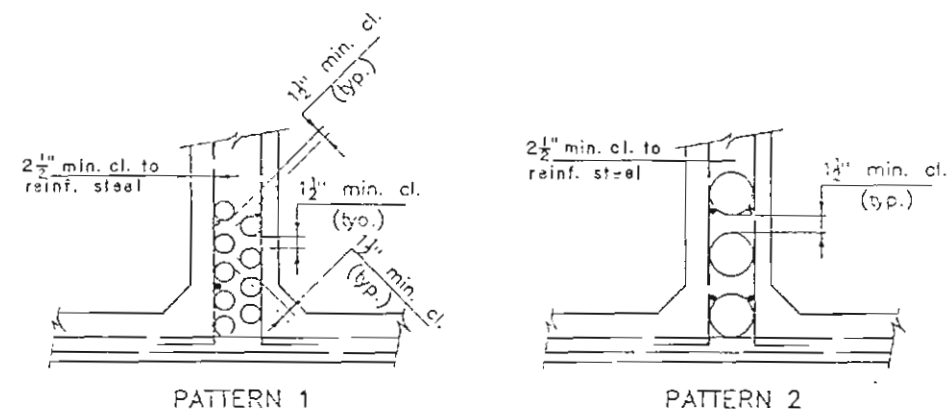
SUPERSTRUCTURE PLACING SCHEDULE

Note: Numbers ①, ② and ③ indicate sequence of placing bottom slab and web concrete when each section constitutes a separate pour.
④ may be placed continuously or in parts, as indicated above and as approved by the Engineer.
Contractor may submit an alternate placing schedule to the Engineer for approval.



Provide #4 ties for each pair of ducts, or single duct. Tie to three stirrups at beginning of flore, and at 1'-6" in all curved portions of webs.

PLAN VIEW OF DUCTS



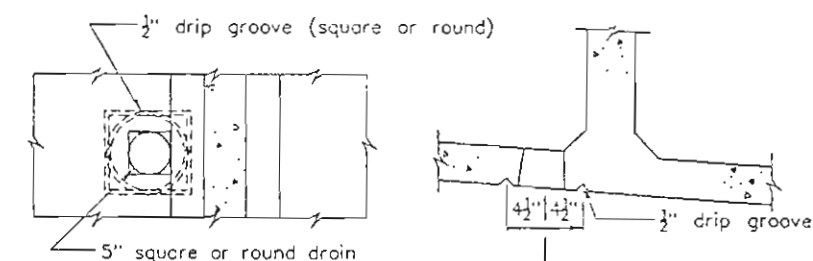
CLEARANCE REQUIREMENTS FOR DUCTS

B-618-2
(use with B-618-1 and B-618-3)

NOTES:

Maintain $1\frac{1}{2}$ " minimum cover over the ducts and a 1" minimum cover over the stirrups.

See abutment and superstructure details for dimensions and reinforcing not shown.



PLAN ELEVATION
BOTTOM SLAB DRAIN DETAIL

Locate at low point of each cell.

REVISED THIS PROJECT

DIVISION OF HIGHWAYS

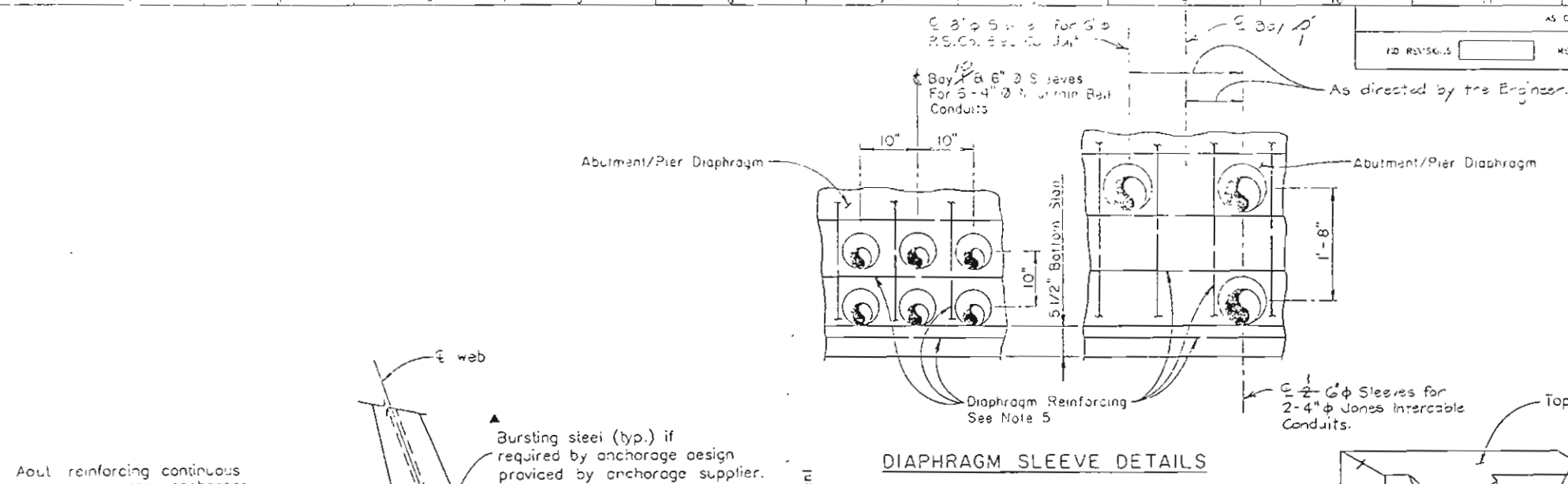
CAST-IN-PLACE POSTTENSIONED BOX GIRDER DETAILS

Designer <i>R. OSMUN</i>	Structure Numbers	<i>F-16-NE</i>
Detailer <i>C. HOWELL</i>		
Drawing Number 813	of 21	Drawings

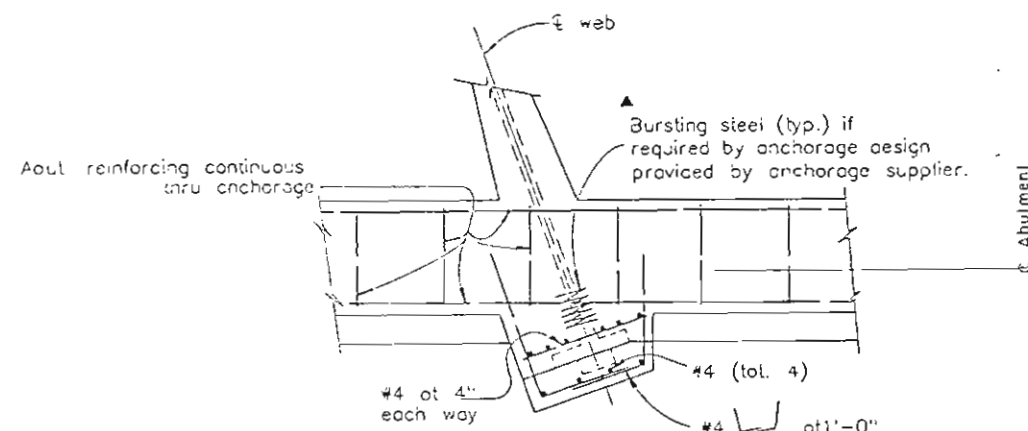
Revision Date: Preliminary Stage on:

DESIGNED BY	DATE	CHECKED BY	DATE
RZO	10/86	RZO	10/87
DESIGNED BY	DATE	CHECKED BY	DATE
E. HAYASHIDA	10/86	E. HAYASHIDA	10/87

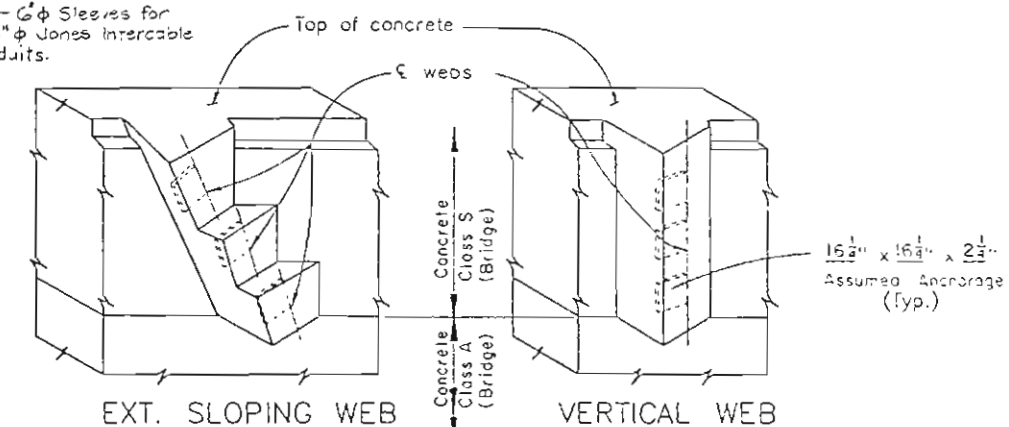
01/08/97 Duro (HEATH PROJ)R-618.3



DIAPHRAGM SLEEVE DETAILS

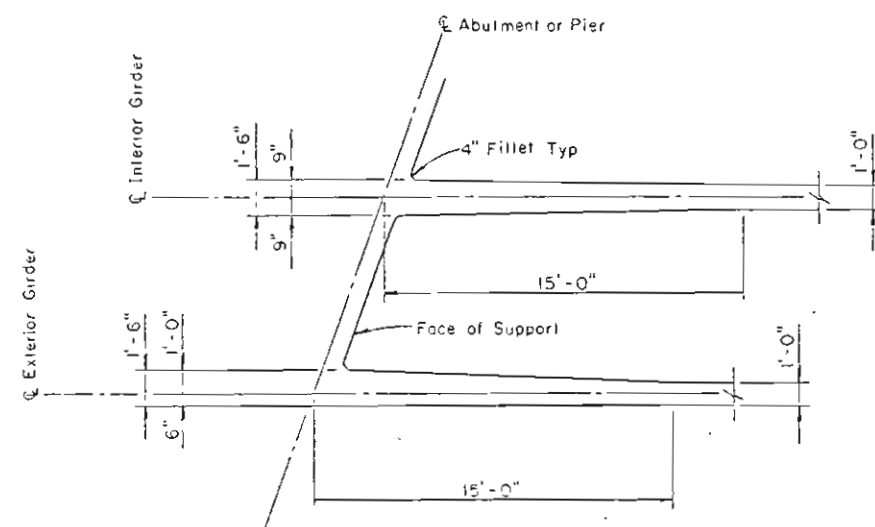


TYPICAL PRESTRESSED ANCHORAGE



TYPICAL ANCHORAGE ILLUSTRATIONS

Details may be modified to specific anchorage.



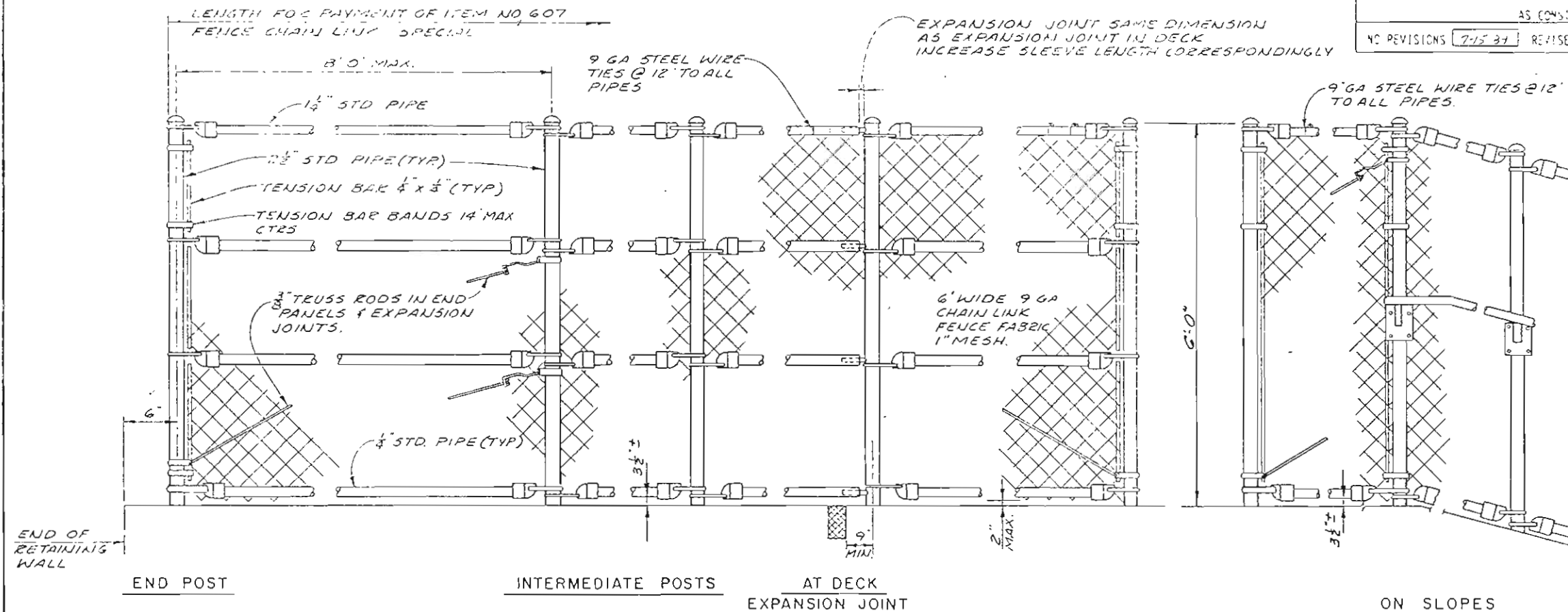
TYPICAL GIRDER FLARE AT ABUTMENTS & PIER

NOTES

1. THE ANCHORAGE SHALL BE COVERED WITH CONCRETE TO PROVIDE A MINIMUM OF 4" OF COVER.
2. THE DISTANCE BETWEEN THE EDGE OF A BEARING PLATE AND THE EDGE OR CORNER OF THE CONCRETE SHALL BE 5" MINIMUM.
3. THE COST FOR ALL REINFORCING DESIGNATED $\blacktriangle$, AND THE COST FOR CONCRETE IN FLARES NOT INCLUDED IN EXPLICIT DETAILS, SHALL NOT BE INCLUDED IN THE BID PRICE FOR CONCRETE AND REINFORCING, BUT SHALL BE INCLUDED IN THE B.I.D. PRICE FOR THE PRESTRESSING STEEL.
4. SEE ABUTMENT DETAILS AND SUPERSTRUCTURE DETAILS FOR DIMENSIONS AND REINFORCING NOT SHOWN.
5. MINOR ADJUSTMENTS TO LOCATION OF REINFORCING STEEL AND/OR SLEEVES MAY BE MADE TO AVOID INTERFERENCES BETWEEN SAID REINFORCING STEEL AND SLEEVES.

DIVISION OF HIGHWAYS			
CAST-IN-PLACE POSTTENSIONED BOX GIRDER DETAILS			
Designer <i>R. OSHULL</i>	Structure	F-16-NE	
Detailer <i>E. HAYASHIDA</i>	Numbers		
Drawing Number B14	of	21	Drawings
Revision Dates		(Preliminary Stage Only)	

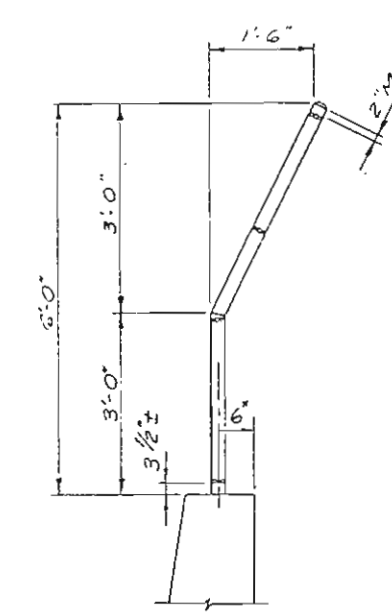
LENGTH FOR PAYMENT OF ITEM NO. 607
FENCE CHAIN LINK SPECIAL



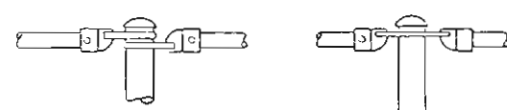
NO. REVISIONS	DATE	BY	REASON	APPROVED
40	7-15-37		REVISED	
			VOID	

AS CONSTRUCTED

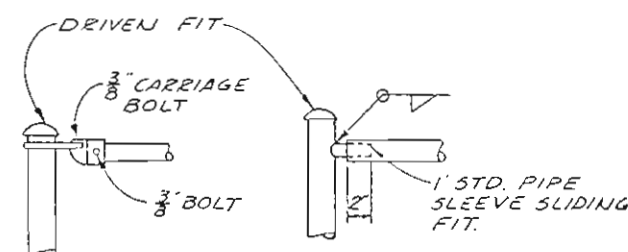
NO.	REVISIONS	DATE	BY	REASON
VIII.				



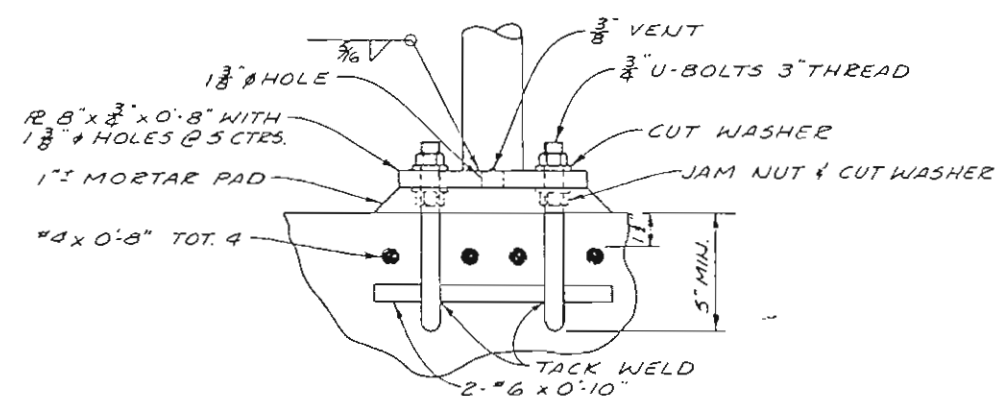
TYPICAL SECTION



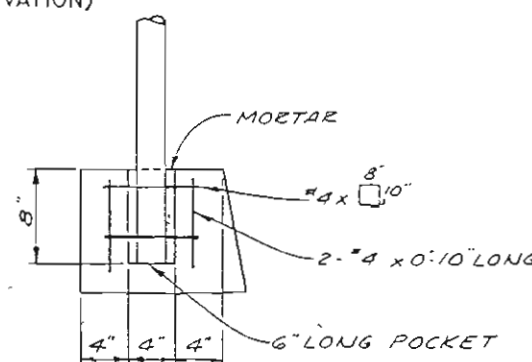
ALTERNATIVE DETAILS



TYPICAL CONNECTION DETAILS



ANCHORAGE DETAIL
(ELEVATION)



ALTERNATIVE ANCHORAGE DETAIL
(SECTION)

NOTES:

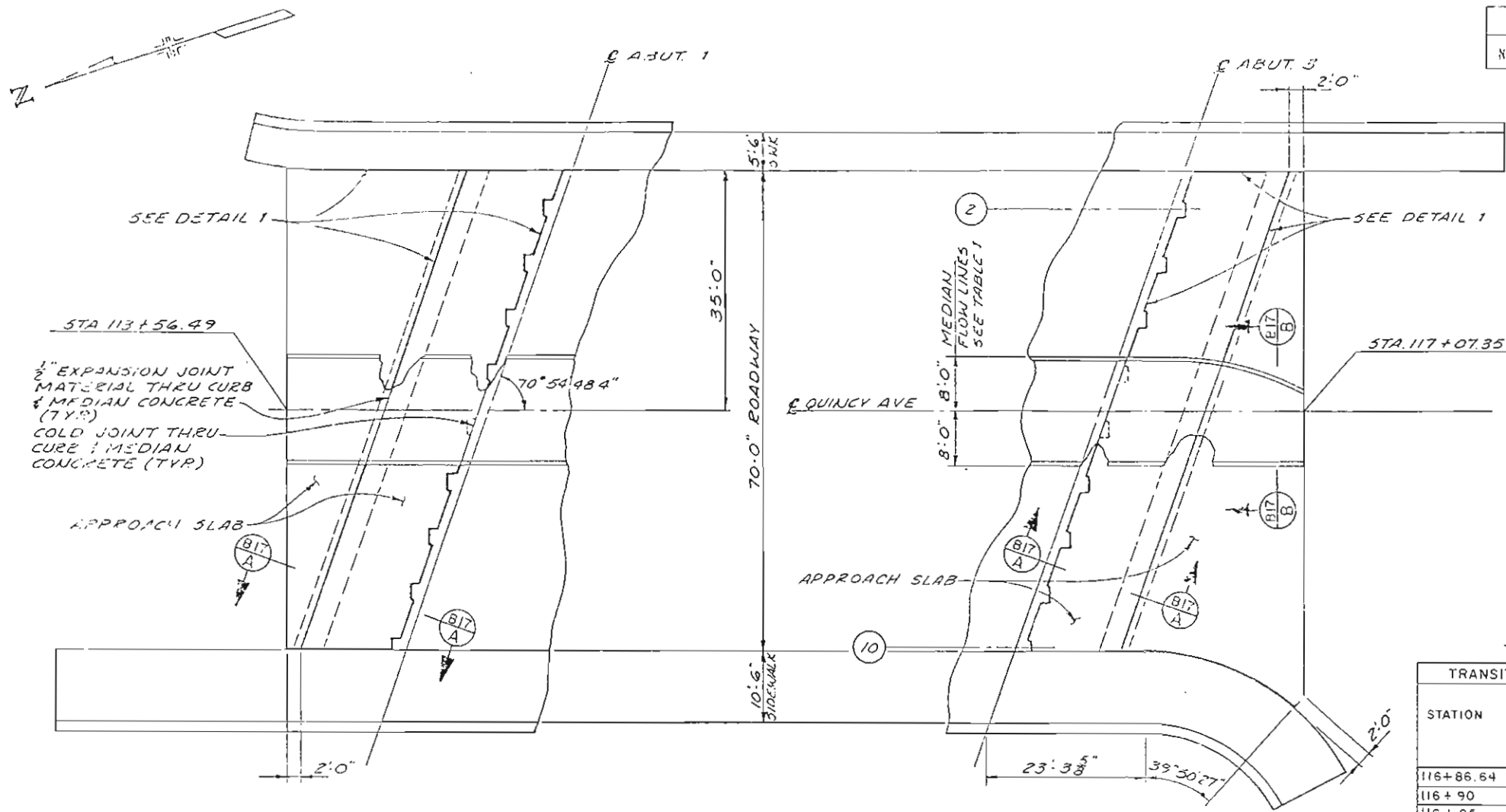
1. RAILING ASSEMBLY EXCEPT CHAIN LINK FABRIC WHICH IS VINYL COATED SHALL BE GALVANIZED AFTER FABRICATION.
2. PEEN ALL 3/8" BOLTS.
3. RAILING SHALL CONFORM TO HORIZONTAL AND VERTICAL ALIGNMENT. POSTS SHALL BE VERTICAL. TOP AND BOTTOM PIPES SHALL BE BENT IF RADIUS IS 100' OR LESS & MAY BE ON B' CHORDS IF RADIUS IS GREATER THAN 100'.
4. ALTERNATIVE DETAILS MAY BE SUBMITTED BY CONTRACTOR FOR ENGINEER'S APPROVAL.

CENTENNIAL ENGINEERING, INC. ARVADA, COLORADO

DIVISION OF HIGHWAYS

FENCE CHAIN LINK SPECIAL

Designer R OSMUN	Structure F-16-NE
Detailer G HIRSCH	Numbers
Drawing Number 815	of 21 Drawings



AS CONSTRUCTED			FEDERAL ROAD DISTRICT NO.	DIVISION	PROJECT NO.	NO. OF SHEETS	TOTAL SHEETS
NO REVISIONS	7-15-51	REVISED	VOID	VIII	COLORADO	IXFU 470-1(55)	3-7 49
REVISIONS							

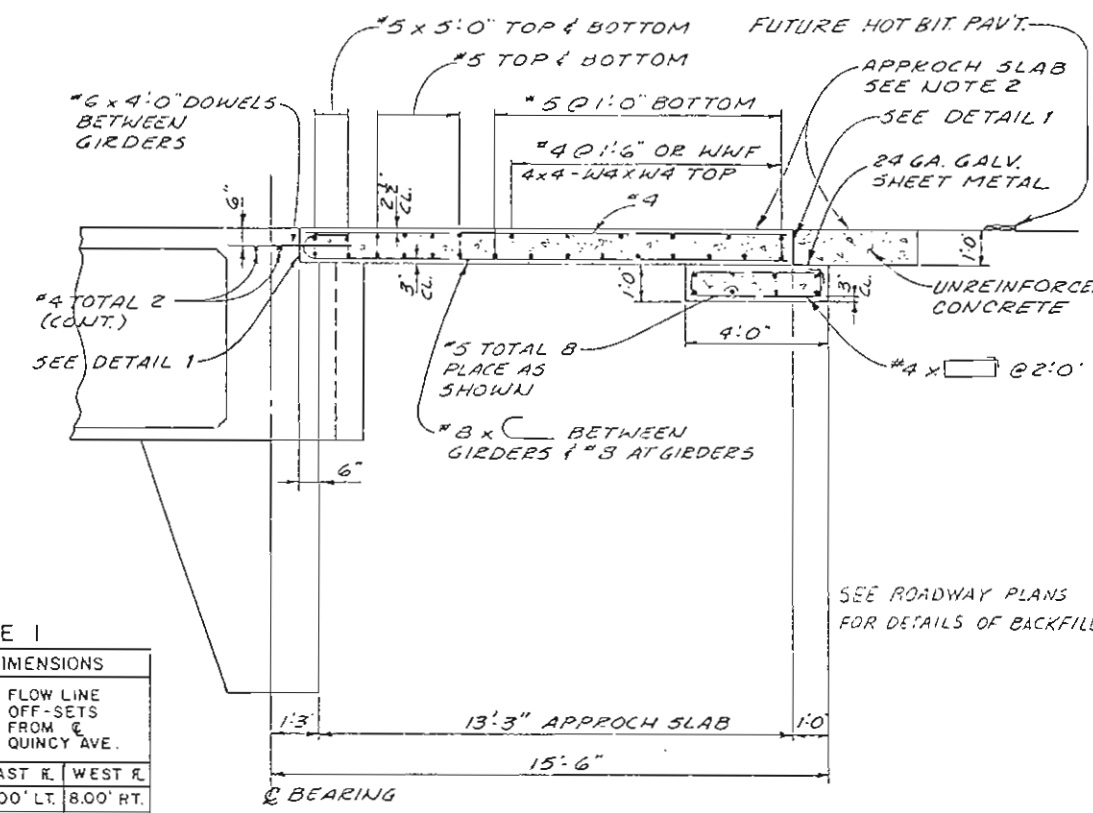
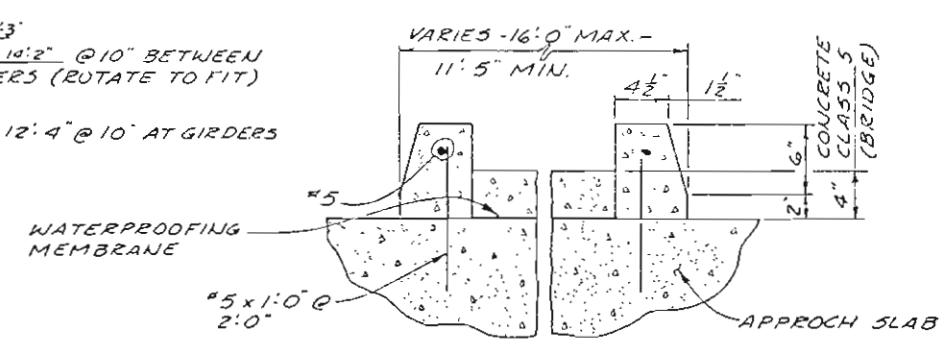
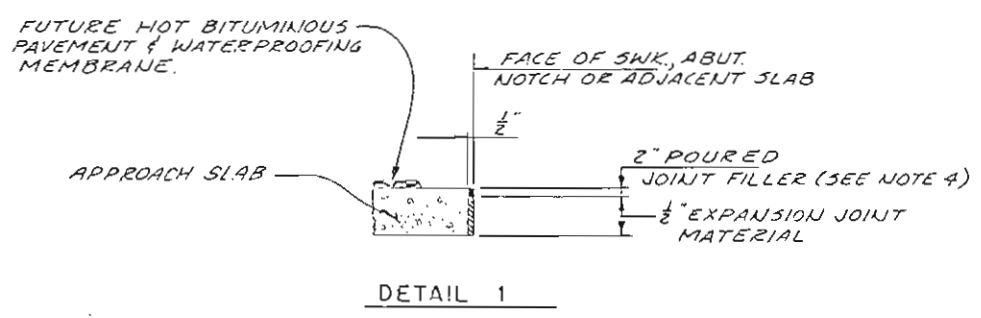
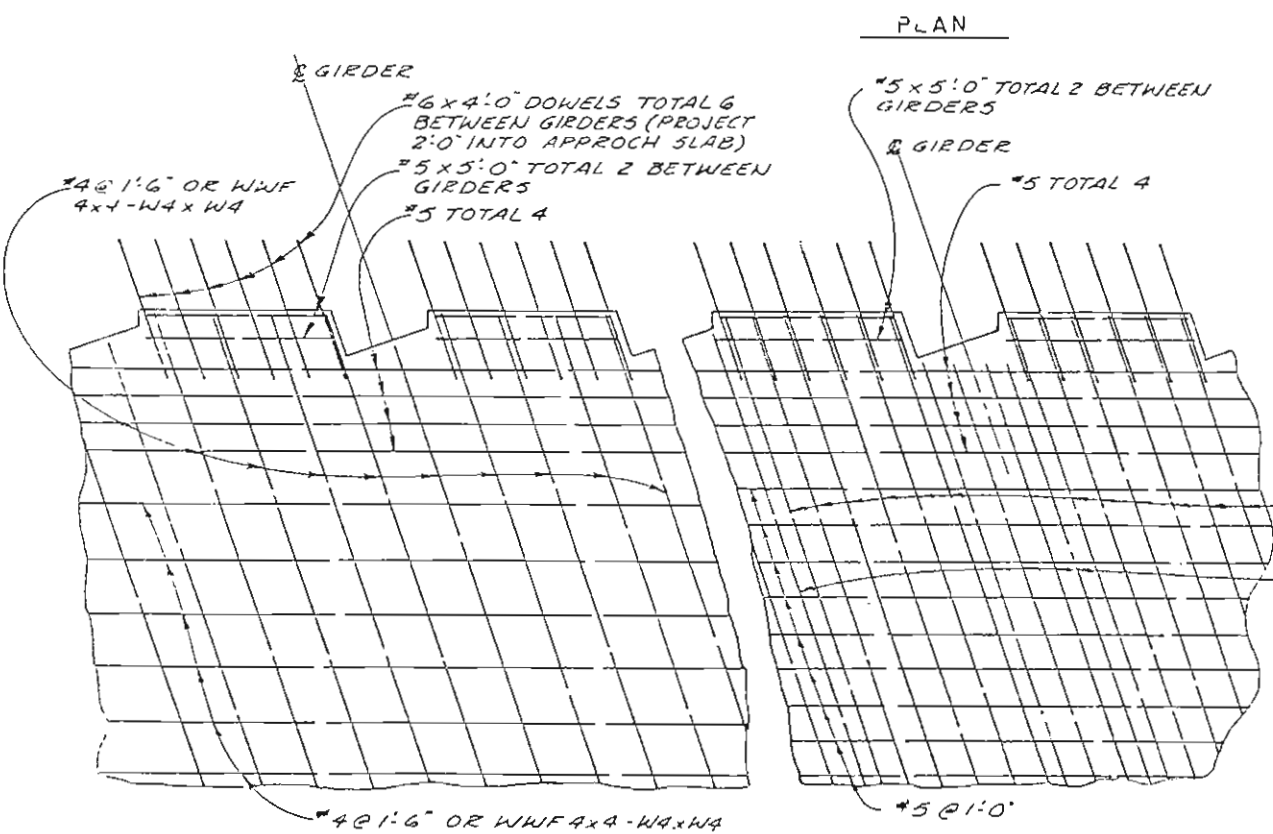


TABLE 1
TRANSITION DIMENSIONS

STATION	FLOW LINE OFF-SETS FROM QUINCY AVE.	
	EAST R.	WEST R.
116+86.64	8.00' LT.	8.00' RT.
116+90	7.88' LT.	8.00' RT.
116+95	7.28' LT.	8.00' RT.
117+00	6.14' LT.	8.00' RT.
117+05	4.43' LT.	8.00' RT.
117+07.35	3.41' LT.	8.00' RT.



- NOTES:
1. USE 2" COVER FOR PRIMARY REINFORCEMENT AND 1 1/2" COVER FOR TIES UNLESS OTHERWISE NOTED.
 2. CONCRETE FOR APPROACH SLABS (INCLUDING REINFORCED & UNREINFORCED CONCRETE) SHALL BE CONCRETE CLASS 5 (BRIDGE).
 3. ALL REINFORCING SHOWN ON THIS SHEET SHALL BE EPOXY COATED.
 4. THE COST FOR 2" DEEP POURED JOINT FILLER SHALL BE INCL. IN THE COST FOR CONCRETE CLASS 5 (BRIDGE).
 5. LONGITUDINAL MEDIAN CURB REINFORCEMENT SHALL NOT PASS THRU COLD AND EXPANSION JOINTS.

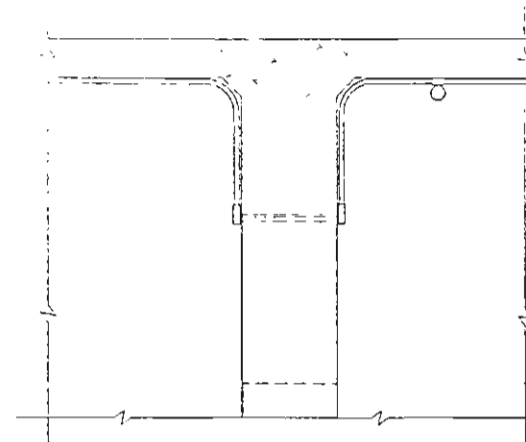
CENTENNIAL ENGINEERING, INC. ARVADA, COLORADO

DIVISION OF HIGHWAYS

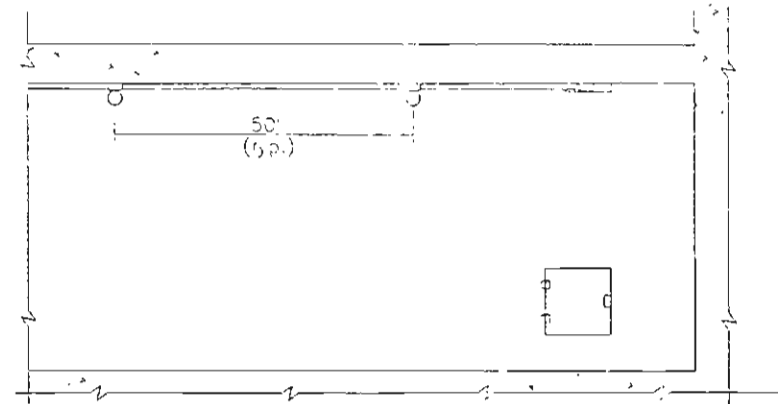
APPROACH SLAB DETAILS

Designer R. OSMUN	Structure F-16-NE
Detainer G. Hirsch	Drawings
Drawing Number B17	of 21

DESIGNED BY	CHECKED BY	DATE	REVISION
RLO	RLO	7-87	1
RLO	RLO	7-87	2
RLO	RLO	7-87	3
RLO	RLO	7-87	4
RLO	RLO	7-87	5
RLO	RLO	7-87	6
RLO	RLO	7-87	7
RLO	RLO	7-87	8
RLO	RLO	7-87	9
RLO	RLO	7-87	10
RLO	RLO	7-87	11
RLO	RLO	7-87	12
RLO	RLO	7-87	13
RLO	RLO	7-87	14
RLO	RLO	7-87	15
RLO	RLO	7-87	16
RLO	RLO	7-87	17
RLO	RLO	7-87	18
RLO	RLO	7-87	19
RLO	RLO	7-87	20
RLO	RLO	7-87	21
RLO	RLO	7-87	22
RLO	RLO	7-87	23
RLO	RLO	7-87	24
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RLO	RLO	7-87	27
RLO	RLO	7-87	28
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RLO	RLO	7-87	43
RLO	RLO	7-87	44
RLO	RLO	7-87	45
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RLO	RLO	7-87	50
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RLO	RLO	7-87	54
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RLO	RLO	7-87	100



SECTION A-A

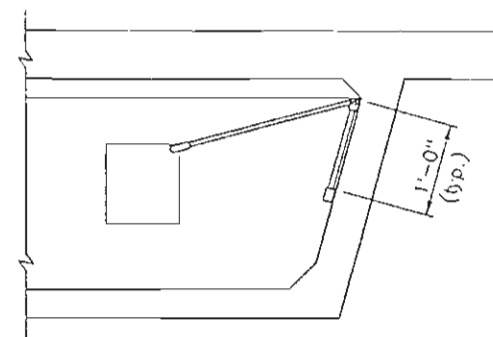


PLAN

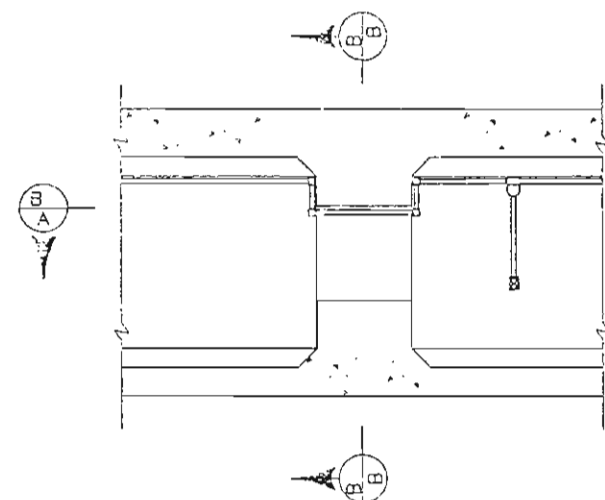
AS CONSTRUCTED	REVISED	7-87	NO. 1
10 PERSONS	REVISED	7-87	NO. 1

DESIGNER	PROJECT NUMBER	SHEET NUMBER
OSMUN	IXFU 470-1(55)	35

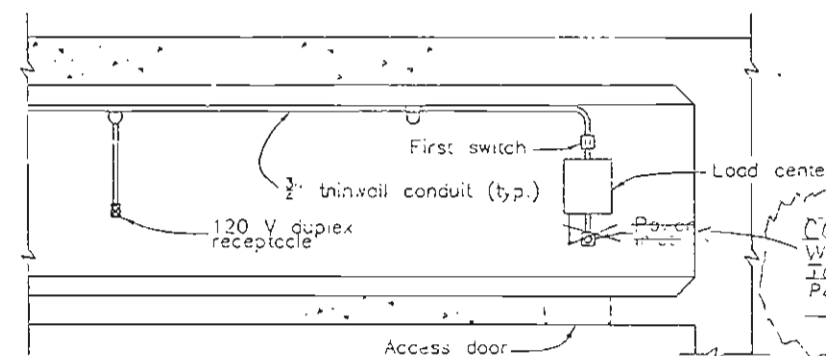
REVISIONS		



SECTION B-B



ELEVATION



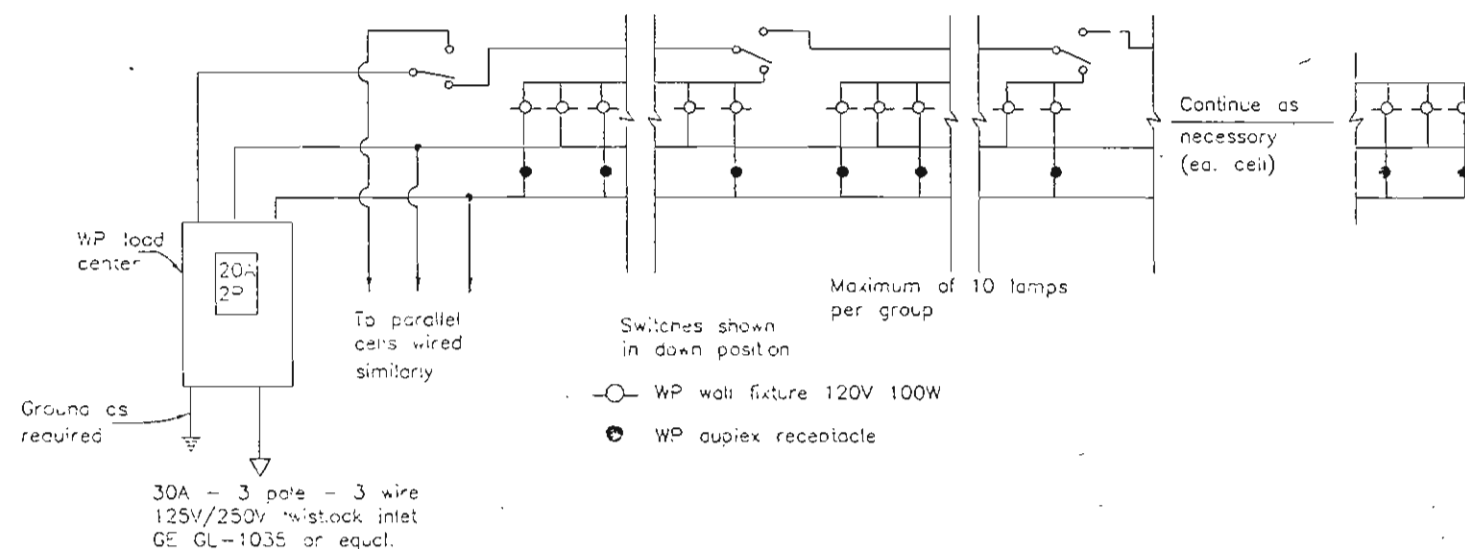
CONDUITS AND WIRING
WERE ADDED TRANSVERSELY
TO ALLOW FOR A FUTURE
PERMANENT POWER SOURCE

Typical outlet box
For use with threaded
tinwall conduit.

Typical wall mounted WP light
fixture with 100W incandescent bulb

Typical single gang box threaded
for tinwall conduit.

FIXTURE DETAILS



WIRING DIAGRAM

NOTE:
All fasteners shall be $\frac{1}{4}$ "
Supports for conduit shall be at 10' spacing (max.).

DIVISION OF HIGHWAYS

INTERIOR LIGHTING
(CONCRETE BOX GIRDER)

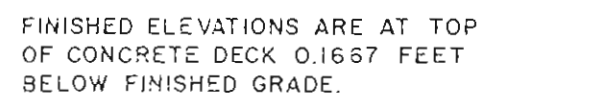
Designer R OSMUN	Structure	F-15-NE
Detailer C. NECESSARY	Numbers	
Drawing Number E 18	of 21	Drawings

Revision Dates (Preliminary, Stage Only)

FEDERAL WORK REGION NO	DIVISION	PROJECT	UNIT NO	TOTAL SHEETS
VIII	COLUMBIA	XFU 470-1(53)	5	41

[illegible]

LONGITUDINAL LINE		CL QUINCY	& PROF GR
AF ARUT 1	113-84.5973	0.0000	5802.0743
CL ARUT 1	113-85.9200	0.0000	5802.1221
F-1	113-93.4760	0.0000	5802.3923
F-2	114-01.0200	0.0000	5802.6591
F-3	114-08.5700	0.0000	5802.9225
F-4	114-16.1200	0.0000	5803.1823
F-5	114-23.5700	0.0000	5803.4346
F-6	114-31.2200	0.0000	5803.6914
F-7	114-38.7700	0.0000	5803.9408
F-8	114-46.3200	0.0000	5804.1867
F-9	114-53.8700	0.0000	5804.4290
F-10	114-61.4200	0.0000	5804.6679
F-11	114-68.9700	0.0000	5804.9033
F-12	114-76.5200	0.0000	5805.1352
F-13	114-84.0700	0.0000	5805.3636
F-14	114-91.6200	0.0000	5805.5885
F-15	114-99.1700	0.0000	5805.8100
F-16	115-06.7200	0.0000	5805.0279
F-17	115-14.2700	0.0000	5805.2424
F-18	115-21.8200	0.0000	5805.4533
F-19	115-29.3700	0.0000	5805.6605
CL PIER 2	115-35.9200	0.0000	5805.8646
F-1	115-43.9700	0.0000	5807.0521
F-2	115-51.0200	0.0000	5807.2364
F-3	115-58.0700	0.0000	5807.4176
F-4	115-65.1200	0.0000	5807.5958
F-5	115-72.1700	0.0000	5807.7710
F-6	115-79.2200	0.0000	5807.9431
F-7	115-86.2700	0.0000	5808.1122
F-8	115-93.3200	0.0000	5808.2782
F-9	116-00.3700	0.0000	5808.4412
F-10	116-07.4200	0.0000	5808.6011
F-11	116-14.4700	0.0000	5808.7580
F-12	116-21.5200	0.0000	5808.9119
F-13	116-28.5700	0.0000	5809.0627
F-14	116-35.6200	0.0000	5809.2105
F-15	116-42.6700	0.0000	5809.3552
F-16	116-49.7200	0.0000	5809.4969
F-17	116-56.7700	0.0000	5809.6355
F-18	116-63.8200	0.0000	5809.7711
F-19	116-70.8700	0.0000	5809.9037
CL ARUT 3	116-77.9200	0.0000	5810.0332
BF ARUT 3	116-79.2427	0.0000	5810.0571



DIVISION OF HIGHWAYS

4-3527

BRIDGE DECK ELEVATIONS

10

Designer	R USMUN	Structure	F-16-NE
Updater	C A E TESSARD	Remarks	

Drawing Number	B 19	of	21	Drawings
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ORIGINAL REVISION NO.	DATE	PROJECT	SHEET NO.	TOTAL SHEETS
VIII	11/10/100	IXFU 470-1(55)	57	41

REVISION		

AS CONSTRUCTED		
NO REVISIONS	REVISED	VOID

7/8"

LEFT (EAST)
OR RIGHT
(WEST) EDGE
OF DECK

FINISHED ELEVATIONS ARE AT TOP
OF CONCRETE DECK 0.1667 FEET
BELOW FINISHED GRADE.

CENTENNIAL ENGINEERING, INC. ARVADA, COLORADO

DIVISION OF HIGHWAYS

BRIDGE DECK ELEVATIONS

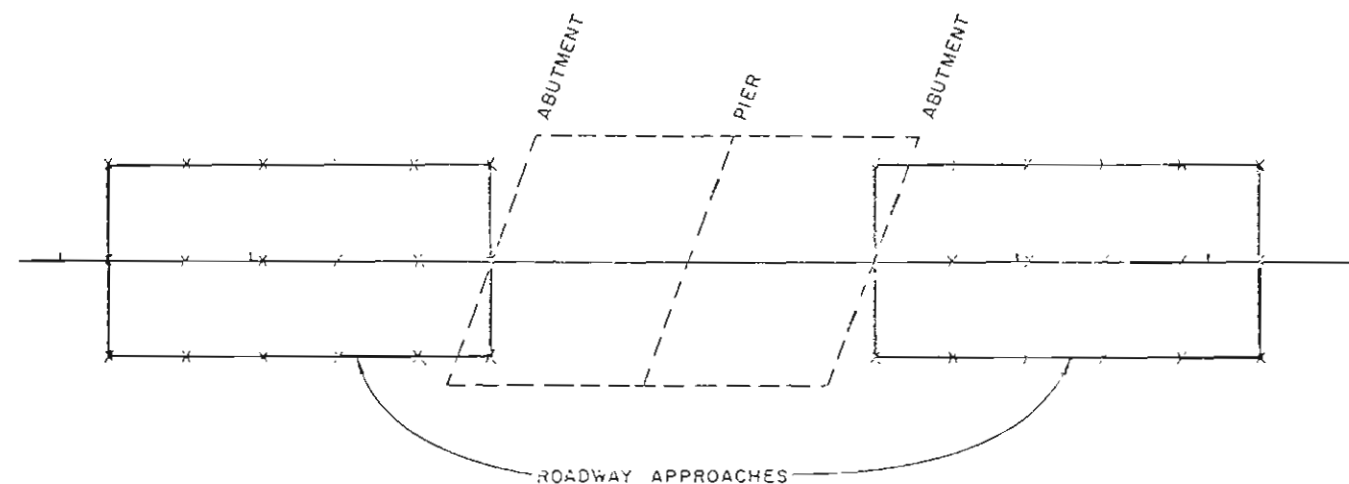
Designed by P. OSMUN	Structure Number	F-16-NE
Checked by C. NECESSARY		
Drawing Number 320	of	21 Drawings

ROADWAY APPROACHES

ELEVATIONS ARE AT FINISHED GRADE

STATION:	ELEVATION ON ROADWAY APPROACHES (AT FINISHED GRADE)				
	-33.0000	-9.0000	0.0000	9.0000	33.0000
112+50	5726.4720	5794.9520	5797.1320	5794.9520	5795.4720
112+60	5727.8543	5797.3343	5797.5143	5797.3343	5798.8543
112+70	5727.2066	5797.7166	5797.8966	5797.7166	5797.2366
112+80	5757.6149	5798.0989	5798.2789	5798.0989	5797.6149
112+90	5798.0012	5798.4812	5798.6612	5798.4812	5798.0012
113+00	5798.3835	5798.8635	5799.0435	5798.8635	5798.3835
113+10	5798.7658	5799.2458	5799.4258	5799.2458	5798.7658
113+20	5799.1481	5799.6281	5799.8081	5799.6281	5799.1481
113+30	5799.5304	5800.0104	5800.1904	5800.0104	5799.5304
113+40	5799.9127	5800.3927	5800.5727	5800.3927	5799.9127
113+50	5800.2950	5800.7750	5800.9550	5800.7750	5800.2950
113+60	5800.6772	5801.1572	5801.3372	5801.1572	5800.6772
113+70	5801.0594	5801.5394	5801.7194	5801.5394	5801.0594
113+80	5801.4417	5801.9217	5802.1017	5801.9217	5801.4417
113+90	5801.8240	5802.3040	5802.4840	5802.3040	5801.8240
114+00	5802.2063	5802.6863	5802.8663	5802.6863	5802.2063

STATION:	ELEVATION ON ROADWAY APPROACHES (AT FINISHED GRADE)				
	-33.0000	-9.0000	0.0000	9.0000	33.0000
116+70	5809.3941	5809.8741	5810.0541	5810.0842	5810.1791
116+80	5809.5775	5810.0575	5810.2375	5810.2928	5810.4439
116+90	5809.7609	5810.2409	5810.4147	5810.4932	5810.7025
117+00	5809.9443	5810.4243	5810.5858	5810.6865	5810.9560
117+10	5810.1277	5810.6077	5810.7507	5810.8736	5811.2013
117+20	5810.3111	5810.7911	5810.9096	5811.0547	5811.4416
117+30	5810.4945	5810.9745	5811.0623	5811.2296	5811.6757
117+40	5810.6779	5811.1579	5811.2089	5811.3924	5811.9037
117+50	5810.8613	5811.3413	5811.3499	5811.5611	5812.1236
117+60	5811.0447	5811.5247	5811.4838	5811.7176	5812.3414
117+70	5811.2281	5811.7081	5811.6120	5811.8681	5812.5510
117+80	5811.4115	5811.8915	5811.7341	5812.0124	5812.7545
117+90	5811.5949	5812.0749	5811.8501	5812.1482	5812.9432
118+00	5811.7783	5812.2583	5811.9600	5812.2735	5813.1094
118+10	5811.9617	5812.4417	5812.0637	5812.3882	5813.2533
118+20	5812.1451	5812.6251	5812.1614	5812.4923	5813.3748



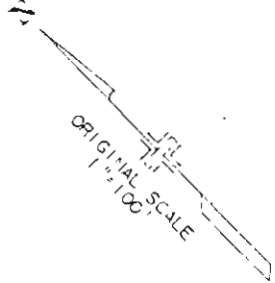
THIS DRAWING IS SCHEMATIC ONLY AND DOES NOT NECESSARILY REFLECT THE ACTUAL BRIDGE LAYOUT.

CENTENNIAL ENGINEERING, INC. ARVADA, COLORADO

DIVISION OF HIGHWAYS

ROADWAY APPROACHES

QUINCY INTERCHANGE
DETAILS

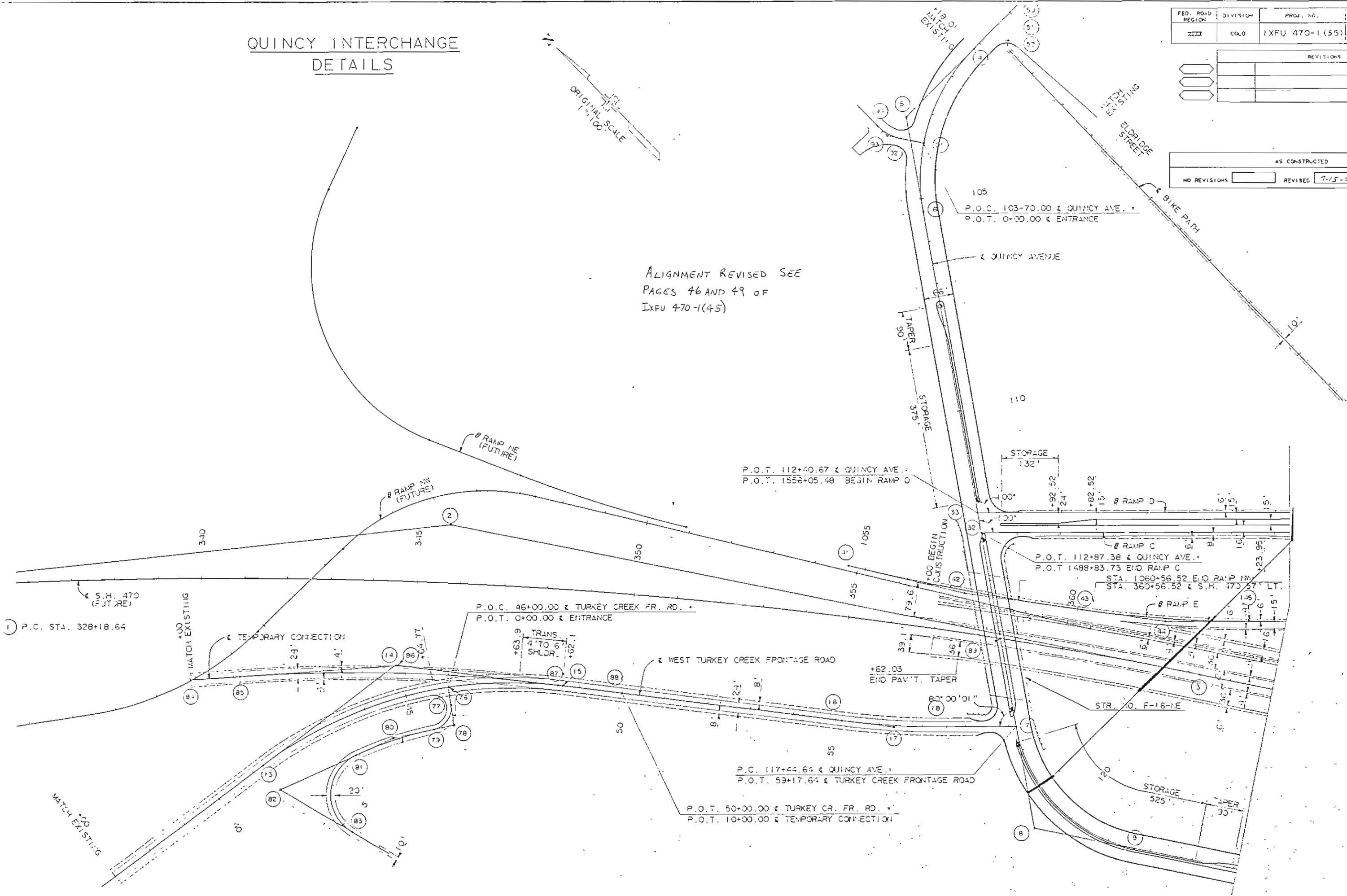


FED. ROAD REGION	DIVISION	PROJ. NO.	SHEET NO.	TOTAL SHEETS
XIII	COLO.	IXFU 470-1(55)	39	49

REVISIONS		

AS CONSTRUCTED		
NO REVISIONS	REVISED 7-15-95	VOID

ALIGNMENT REVISED SEE
PAGES 46 AND 49 OF
IXFU 470-1(45)



PROJECT CONTROL

AS CONSTRUCTED
REVISIONS
VOID

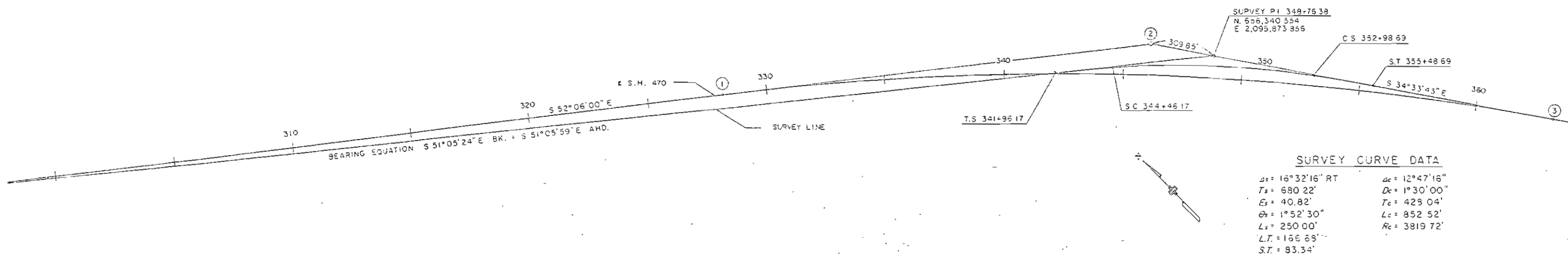
FED. ROAD REGION	DIVISION	PROJECT NO.	SHEET NO.	TOTAL SHEETS
XXXX	00.0	1XFU 470-1 (55)	40	49

REV. SIGNS

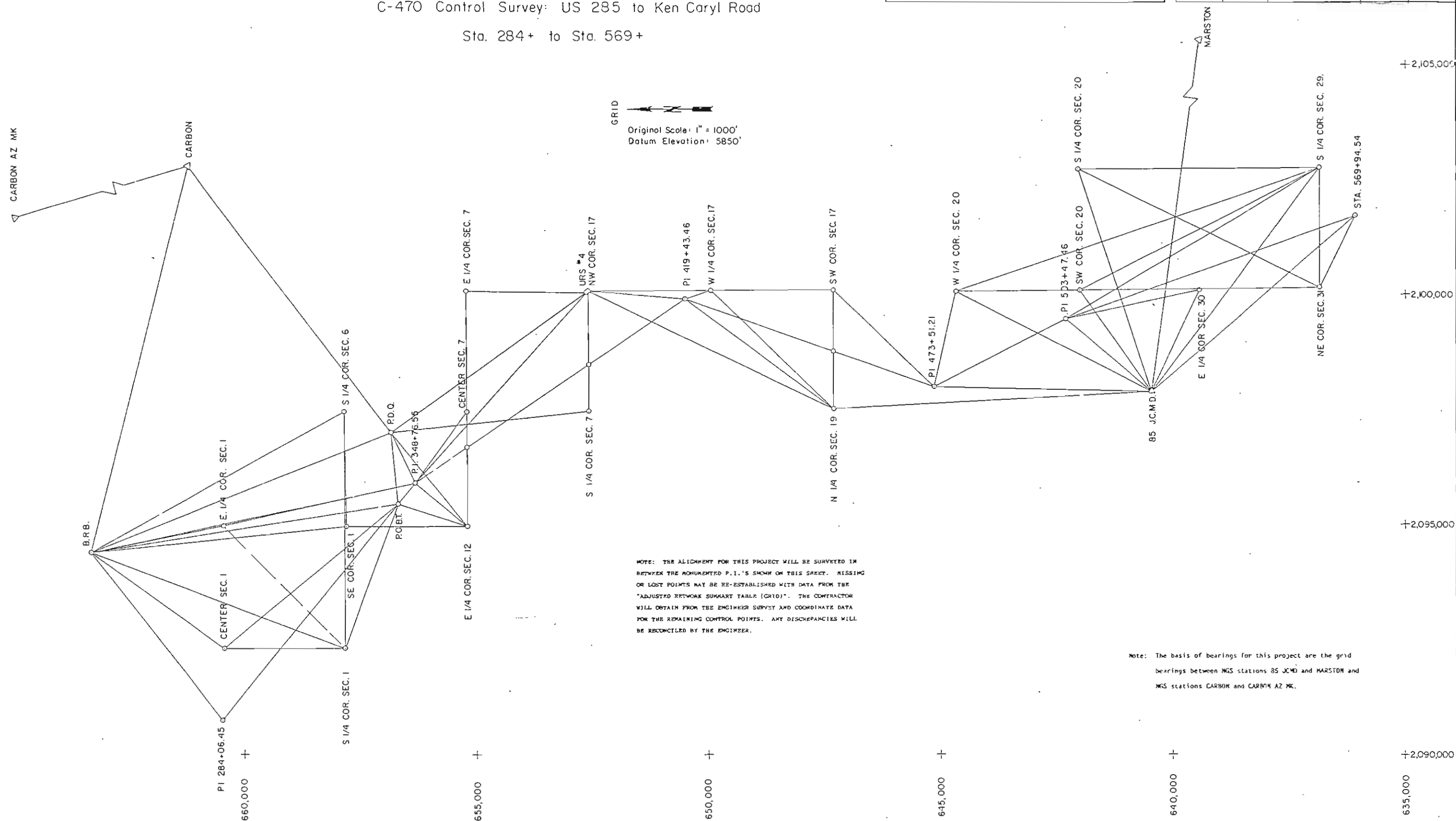
PNT NO.	POINT DESCRIPT.	BASELINE	CURVE DATA						
			Δ	D	T	L	R	S.E.	D.S.
1	P.C.	324+4.64							
2	P.T.	345+46.27	17°32'17" RT	0°40'	1,767.63'	3,507.61'	11,453.16'	0.02	70 M.P.H.
3	P.C.	363+26.25							
4	P.T.	101+18.01							
5	P.C.	103+21.48	54°55'05" LT	13°19'29"	223.45'	412.15'	430.00'	0.035	30 M.P.H.
6	P.T.	105+30.18							
7	P.C.	117+44.64							
8	P.T.	59+17.64							
9	P.C.	119+53.12	68°32'17" LT	16°22'13"	238.43'	418.68'	350.00'	0.037	30 M.P.H.
10	P.T.	121+53.32							
11	P.C.	138+00.54							
12	P.T.	140+58.40	44°06'00"	9°	257.95'	490.00'	635.62'	0.04	40 M.P.H.
13	P.C.	142+30.54							
14	P.T.	41+28.35							
15	P.C.	45+14.4	44°01'31" RT	6°	385.06'	733.75'	954.93'	0.077	50 M.P.H.
16	P.T.	48+52.10							
17	P.C.	55+13.18							
18	P.T.	56+35.17	7°15'00" LT	3°	120.99'	241.57'	1,909.85'	R.C.	30 M.P.H.
19	P.C.	57+55.95							
20	P.T.	265+30.00							
21	P.C.	270+39.34							
22	P.T.	1274+55.98							
23	P.C.	1275+36.81	86°33'47" RT	24°54'40"	215.50'	347.49'	230.00'	0.04	25 M.P.H.
24	P.T.	1276+83.95							
25	P.C.	133+52.10							
26	P.T.	134+17.10							
27	P.C.	1372+73.42							
28	P.T.	373+59.34							
29	P.C.	375+91.17	55°15'07" RT	24°54'40"	120.37'	221.80'	230.00'	0.04	25 M.P.H.
30	P.T.	1377+37.50							
31	P.C.	1379+75.00	34°44'54" RT	12°14'34"	145.43'	283.33'	458.00'	0.08	40 M.P.H.
32	P.T.	1463+38.52							
33	P.C.	1476+73.95	19°27'45" LT	27°56'57"		685.04'	206.00'	0.08	27 M.P.H.
34	P.T.	1488+33.73							
35	P.C.	112+87.38							
36	P.T.	112+50.67							
37	P.C.	556+26.48							
38	P.T.	558+01.37							
39	P.C.	572+11.42	56°13'33" RT	16°16'35"	188.05'	345.43'	352.00'	.08	35 M.P.H.
40	P.T.	1571+66.50							
41	P.C.	573+48.43							
42	P.T.	1574+76.22							
43	P.C.	575+40.11	34°15'47" LT	16°16'36"	110.19'	213.57'	353.00'	.08	35 M.P.H.
44	P.T.	581+17.30							
45	P.C.	576+12.35							
46	P.T.	65+42.00							
47	P.C.	65+42.00							
48	P.T.	65+42.00							
49	P.C.	65+42.00							
50	P.T.	1622+07.38	5°30'00" LT	1°00'00"	275.21'	550.00'	5729.58'	.021	50 M.P.H.
51	P.C.	65+42.00							
52	P.T.	65+42.00	7°52'15" LT	2°00'00"	137.08'	393.54'	2864.79'	.038	50 M.P.H.
53	P.C.	65+42.00							
54	P.T.	65+42.00							
55	P.C.	65+42.00							
56	P.T.	1667+34.57	16°16'38" RT	5°00'00"	163.88'	325.54'	1145.92'	.071	50 M.P.H.
57	P.C.	1669+56.24							
58	P.T.	1688+10.30							
59	P.C.	100+50.3							
60	P.T.	100+50.3							
61	P.C.	100+50.3							
62	P.T.	100+50.3							
63	P.C.	100+50.3							
64	P.T.	100+50.3							
65	P.C.	100+50.3							
66	P.T.	100+50.3							
67	P.C.	100+50.3							
68	P.T.	100+50.3							
69	P.C.	100+50.3							
70	P.T.	100+50.3							
71	P.C.	100+50.3							
72	P.T.	100+50.3							
73	P.C.	100+50.3							
74	P.T.	100+50.3							
75	P.C.	100+50.3							
76	P.T.	100+50.3							
77	P.C.	100+50.3							
78	P.T.	100+50.3							
79	P.C.	100+50.3							
80	P.T.	100+50.3							
81	P.C.	100+50.3							
82	P.T.	100+50.3							
83	P.C.	100+50.3							
84	P.T.	100+50.3							
85	P.C.	100+50.3							
86	P.T.	100+50.3							
87	P.C.	100+50.3							
88	P.T.	100+50.3							
89	P.C.	100+50.3							
90	P.T.	100+50.3							
91	P.C.	100+50.3							
92	P.T.	100+50.3							
93	P.C.	100+50.3							
94	P.T.	100+50.3							
95	P.C.	100+50.3							
96	P.T.	100+50.3							
97	P.C.	100+50.3							
98	P.T.	100+50.3							
99	P.C.	100+50.3							
100	P.T.	100+50.3							

PNT NO.	POINT DESCRIPT.	BASELINE	CURVE DATA						
			Δ	D	T	L	R	S.E.	D.S.
91	P.C.	103+70.00							
92	P.T.	0+00.00							
93	P.C.	0+54.92							
94	P.T.	0+86.14	34°40'25" RT	57°17'45"	31.22'	60.52'	100.00'		
95	P.C.	1+15.44							
96	P.T.	46+00.00							
97	P.C.	0+00.00							
98	P.T.	0+41.31							
99	P.C.	0+88.71							
100	P.T.	1+17.17	86°52'02" RT	11°43'30"	47.34'	75.80'	50.00'	0.04	
101	P.C.	2+13.60							
102	P.T.	3+09.24	12°43'23" LT	6°37'28"	96.43'	192.07'	864.93'	0.02	20 M.P.H.
103	P.C.	5+01.34							
104	P.T.	5+27.41	125°00'01" LT	57°17'45"	192.10'	218.17'	100.00'	0.08	15 M.P.H.
105	P.C.	0+00.00							
106	P.T.	1+14.99							
107	P.C.	4+84.07							
108	P.T.	8+50.86	11°02'17" RT	1°30'	369.08'	735.81'	3,819.72'	0.02	30 M.P.H.
109	P.C.	10+00.00							
110	P.T.	50+00.00							
111	P.C.	358+53.70							
112	P.T.	115+34.92							
113	P.C.	1472+35.90							
114	P.T.	1671+73.42							

PROJECT CONTROL

[illegible][illegible]

Sta. 284+ to Sta. 569+



NOTE: THE ALIGNMENT FOR THIS PROJECT WILL BE SURVEYED IN
BETWEEN THE MONUMENTED P.I.'S SHOWN ON THIS SHEET. MISSING
OR LOST POINTS MAY BE RE-ESTABLISHED WITH DATA FROM THE
"ADJUSTED NETWORK SUMMARY TABLE (GRID)". THE CONTRACTOR
WILL OBTAIN FROM THE ENGINEER SURVEY AND COORDINATE DATA
FOR THE REMAINING CONTROL POINTS. ANY DISCREPANCIES WILL
BE RECONCILED BY THE ENGINEER.

NOTE: The basis of bearings for this project are the grid bearings between NGS stations 85 JCMD and MARSTON and NGS stations CARBON and CARBON AZ MC.

C-470 CONTROL SURVEY: US 285 TO KEN CARYL ROAD ADJUSTED NETWORK SUMMARY TABLE (GRID)

OCCUPIED POINT	AHEAD POINT	DISTANCE	BEARING
BS JCVD	N 1/4 COR 19-SS-69W	6,861.317	N 3 15 34.9 W
N 1/4 COR 19-SS-69W	SW COR 17-SS-69W	2,590.117	N 89 43 21.0 E
SW COR 17-SS-69W	NW COR 17-SS-69W	5,286.553	N 0 20 47.8 W
NW COR 17-SS-69W	S 1/4 COR 7-SS-69W	2,607.860	S 69 39 43.5 W
S 1/4 COR 7-SS-69W	P.O.D.	4,304.042	N 6 9 33.2 W
P.O.D.	CAPBON	7,241.761	N 52 45 17.3 E
N 1/4 COR 19-SS-69W	W 1/4 COR 17-SS-69W	3,700.599	N 44 12 39.1 E
W 1/4 COR 17-SS-69W	NW COR 17-SS-69W	2,642.652	N 0 29 1.1 W
N 1/4 COR 19-SS-69W	NW COR 17-SS-69W	5,880.404	N 25 47 6.0 E
CAPBON	P.O.L.	212.210	N 76 3 7.3 W
CAPBON	B.R.B.	8,625.540	N 76 3 6.8 W
B.R.B.	PI 294+06.45 C470	4,620.770	S 51 59 41.7 W
B.R.B.	CENTER 1-SS-70W	2,554.893	S 35 50 31.1 W
B.R.B.	E 1/4 COR 1-SS-70W	2,911.389	S 11 14 17.1 E
B.R.B.	S 1/4 COR 6-SS-69W	6,243.210	S 29 3 33.8 E
B.P.B.	P.O.B.T.	6,710.504	S 8 53 45.9 E
B.R.B.	SE COR 1-SS-70W	5,528.689	S 5 51 28.0 E
B.R.B.	S 1/4 COR 1-SS-70W	5,856.982	S 20 50 53.2 W
PI 294+06.45 C470	P.O.B.T.	6,024.341	S 51 5 59.2 E
S 1/4 COR 6-SS-69W	S 1/4 COR 1-SS-70W	5,116.905	S 89 43 6.4 W
SE COR 1-SS-70W	E 1/4 COR 12-SS-70W	2,628.200	S 0 0 3.7 W
SE COR 1-SS-70W	S 1/4 COR 6-SS-69W	2,468.539	N 89 0 48.5 E
P.O.D.	PI 345+86.29 C470	1,230.586	S 64 39 24.4 W
S 1/4 COR 1-SS-70W	CENTER 1-SS-70W	2,591.797	N 0 3 49.5 E
S 1/4 COR 1-SS-70W	P.O.B.T.	3,337.950	S 69 45 37.9 E
P.O.D.	URS #4	5,208.737	S 35 35 5.0 E
P.O.D.	B.R.B.	6,967.969	N 21 57 58.0 W
P.O.D.	S 1/4 COR 7-SS-69W	4,304.042	S 6 9 33.2 E
P.O.D.	NW COR 17-SS-69W	5,253.916	S 35 45 3.0 E
P.O.D.	E 1/4 COR 12-SS-70W	2,635.524	S 50 47 35.5 W
P.O.B.T.	P.O.D.	1,567.917	N 83 54 50.7 E
P.O.B.T.	E 1/4 COR 12-SS-70W	1,575.573	S 17 51 29.0 W
P.O.B.T.	CENTER 1-SS-70W	4,881.299	N 39 52 2.5 W
BS JCVD	PI 473+51.68 C470	4,683.459	N 1 4 58.7 E
BS JCVD	PI 503+47.71 C470	2,432.529	N 40 10 0.9 E
BS JCVD	W 1/4 COR 20-SS-69W	4,740.362	N 27 12 59.5 E
BS JCVD	SW COR 20-SS-69W	2,694.232	N 54 13 19.7 E
BS JCVD	S 1/4 COR 20-SS-69W	5,096.074	N 71 42 43.4 E
BS JCVD	E 1/4 COR 30-SS-69W	2,421.208	S 65 16 31.5 E
BS JCVD	S 1/4 COR 29-SS-69W	6,082.120	S 53 10 55.7 E
BS JCVD	STA 563+94.92 C470	5,823.903	S 40 57 32.2 E
PI 473+51.68 C470	PI 419+43.77 C470	5,703.312	N 19 34 37.9 E
URS #4	E 1/4 COR 7-SS-69W	2,607.280	N 0 39 37.7 E
PI 345+86.29 C470	P.O.B.T.	574.133	N 51 8 6.5 W
PI 345+86.29 C470	CENTER 7-SS-69W	1,917.742	S 54 14 1.0 E
PI 345+86.29 C470	URS #4	5,560.987	S 48 9 44.8 E
PI 345+86.29 C470	NW COR 17-SS-69W	5,608.293	S 48 12 50.5 E
PI 345+86.29 C470	E 1/4 COR 12-SS-70W	1,470.651	S 39 13 35.9 W
PI 345+86.29 C470	S 1/4 COR 1-SS-70W	3,886.260	N 67 3 5.2 W
E 1/4 COR 7-SS-69W	CENTER 7-SS-69W	2,617.215	S 89 34 39.3 W
CENTER 7-SS-69W	E 1/4 COR 12-SS-70W	2,486.161	S 89 34 37.1 W
STA 563+94.92 C470	NE COR 31-SS-69W	1,722.263	N 64 9 55.3 W
S 1/4 COR 23-SS-69W	NE COR 31-SS-69W	2,501.250	S 89 57 0.0 W
E 1/4 COR 23-SS-69W	NE COR 31-SS-69W	2,635.325	S 1 29 22.5 E
E 1/4 COR 20-SS-69W	PI 503+47.71 C470	2,939.996	N 12 23 43.3 W
S 1/4 COR 20-SS-69W	S 1/4 COR 29-SS-69W	5,244.050	S 0 19 52.9 E
S 1/4 COR 20-SS-69W	NE COR 31-SS-69W	5,942.310	S 26 6 25.9 W
SW COR 20-SS-69W	S 1/4 COR 29-SS-69W	5,869.259	S 27 12 15.0 E
SW COR 20-SS-69W	NE COR 31-SS-69W	5,222.931	S 0 53 56.7 E
PI 503+47.71 C470	STA 563+94.92 C470	6,648.753	S 19 46 3.5 E
PI 503+47.71 C470	S 1/4 COR 29-SS-69W	6,417.226	S 30 56 47.7 E
PI 503+47.71 C470	P.O.L.N.	1,364.161	N 27 40 5.1 W

OCCUPIED POINT	AHEAD POINT	DISTANCE	BEARING
W 1/4 COR 20-SS-69W	S 1/4 COR 29-SS-69W	8,311.509	S 19 57 49.8 E
W 1/4 COR 20-SS-69W	NE COR 31-SS-69W	7,863.235	S 0 43 36.7 E
PI 473+51.68 C470	P.O.L.S.	573.407	S 27 40 5.4 E
PI 473+51.68 C470	W 1/4 COR 20-SS-69W	2,131.236	S 77 20 26.4 E
SW COR 17-SS-69W	PI 473+51.68 C470	3,032.235	S 46 7 56.7 W
STA 563+94.92 C470	POL 561+95.92 C470	793.159	N 19 46 3.4 W
POL 561+95.92 C470	NE COR 31-SS-69W	1,279.634	S 23 57 3.6 W
P.O.L.N.	P.O.L.S.	1,250.772	N 27 40 5.2 W
PI 419+43.77 C470	URS #4	2,121.003	N 3 30 31.4 E
PI 419+43.77 C470	NW COR 17-SS-69W	2,036.023	N 4 36 30.3 E
PI 419+43.77 C470	W 1/4 COR 17-SS-69W	585.443	S 19 0 44.5 E
PI 419+43.77 C470	N 1/4 COR 19-SS-69W	3,998.656	S 36 42 0.8 W
URS #4	NW COR 17-SS-69W	47.562	S 54 15 40.1 E
NW COR 17-SS-69W	S 1/4 COR 7-SS-69W	2,607.860	S 60 39 40.5 W
PI 419+43.77 C470	POL 362+36.36 C470	5,766.944	N 34 33 26.8 W
POL 362+36.36 C470	E 1/4 COR 7-SS-69W	3,397.126	N 89 34 29.2 E
NW COR 17-SS-69W	STA 394+18.46 C470	1,501.099	S 89 39 43.9 W
S 1/4 COR 7-SS-69W	BL BELLEVUE (C470)	11.413	N 0 5 29.8 W
NW COR 17-SS-69W	STA 394+09.95 C470	1,805.863	S 49 54 43.3 W
N 1/4 COR 19-SS-69W	STA 450+46.44 C470	1,251.053	N 39 49 21.2 E
STA 450+46.44 C470	STA 455+55.68 C470	509.099	S 19 34 36.3 W
STA 455+55.68 C470	STA 437+25.49 C470	1,830.109	N 19 34 36.4 E
STA 437+25.49 C470	STA 450+20.70 C470	1,285.105	S 19 34 36.4 W
STA 450+20.70 C470	BL BOWLES (C470)	1,993.921	N 89 42 36.4 E

SALE STAFF DEVELOPMENT
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MATCH LINE STA. 110+00

FEDERAL BUREAU OF INVESTIGATION U.S. DEPARTMENT OF JUSTICE	DIVISION	PROJECT NO.	SHEET NO.
VIII	COLORADO	IXFU 470-1 (55)	57

ORIGINAL SCALE 1" = 5'

STA. 112-42 RT. TO STA. 112-55 P
REG D- REMOVE 100 L.F. OF FENCE
160

PROPOSED R.O.N.
STATION 112-70 RI. REMOVE STRUCTURE

STR. NO. P-16-N

STA 357+60 BEGIN IXFU 470-1155
STA 357 60 ON IXFU 470-1151) MP 6.1

STA. 361+00 END PROJECT 1XFU47Q-1(55)

SW 1/4 SEC. 7
T-5-S R-63-W

B.M. #359 - EL. 5785-98
CHISELED "D" S.E. FOOTER
POWER TOWER 174' T. 359+40

B.M. #362 - EL. 5799.55
REBAR 199' AT 361-75

AS CONSTRUCTED		
NO REVISIONS	7-15-51	REVISED
		VOID

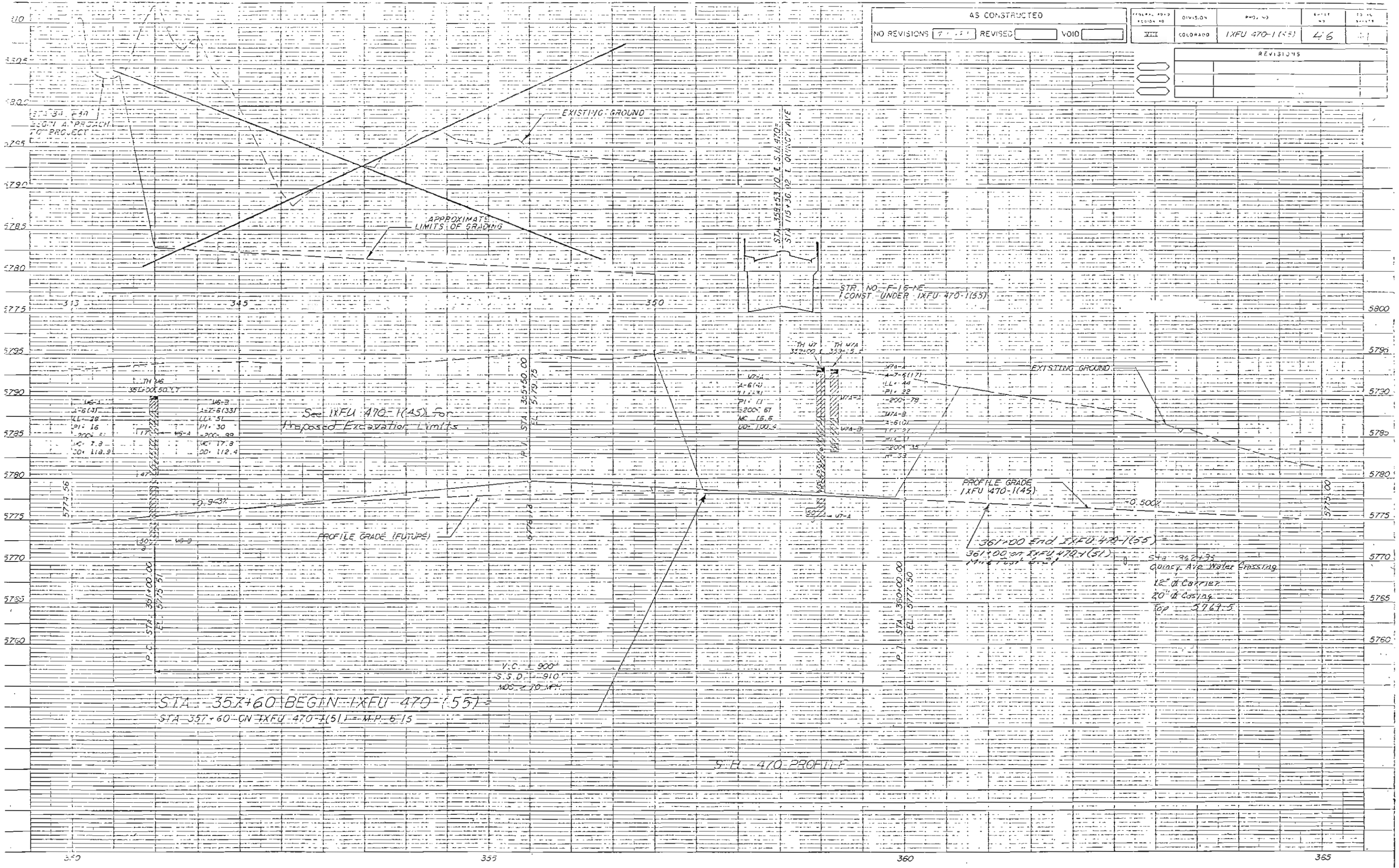
FED. ROAD DISTRICT	DIVISION	PROJ. NO.	SHEET NO.	TOTAL SHEETS
XXX	COLO	IXFU 470-1 (55)	45	49

REVISIONS	

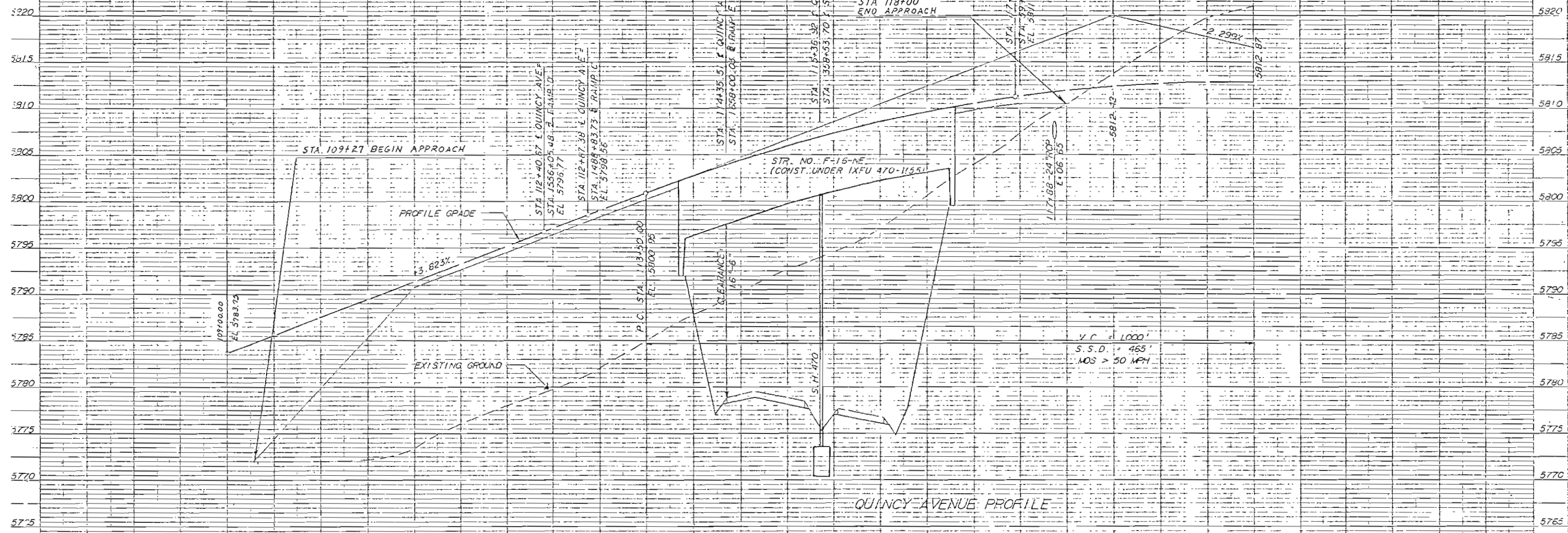


AS CONSTRUCTED		FEDERAL AID	DIVISION	PROJECT NO.	SHEET	DATE
NO REVISIONS	REVISED	III	COLORADO	IXFU 470-1(45)	46	11

REVISIONS	



REVISIONS	



See IXFU 470-1145 For
 Proposed Earthwork Limits

See IXFU 470-1145 For
 Proposed Earthwork Limits

UNCL. EXCAV.
 SUBEXCAVATION
 EMB. X 1.12

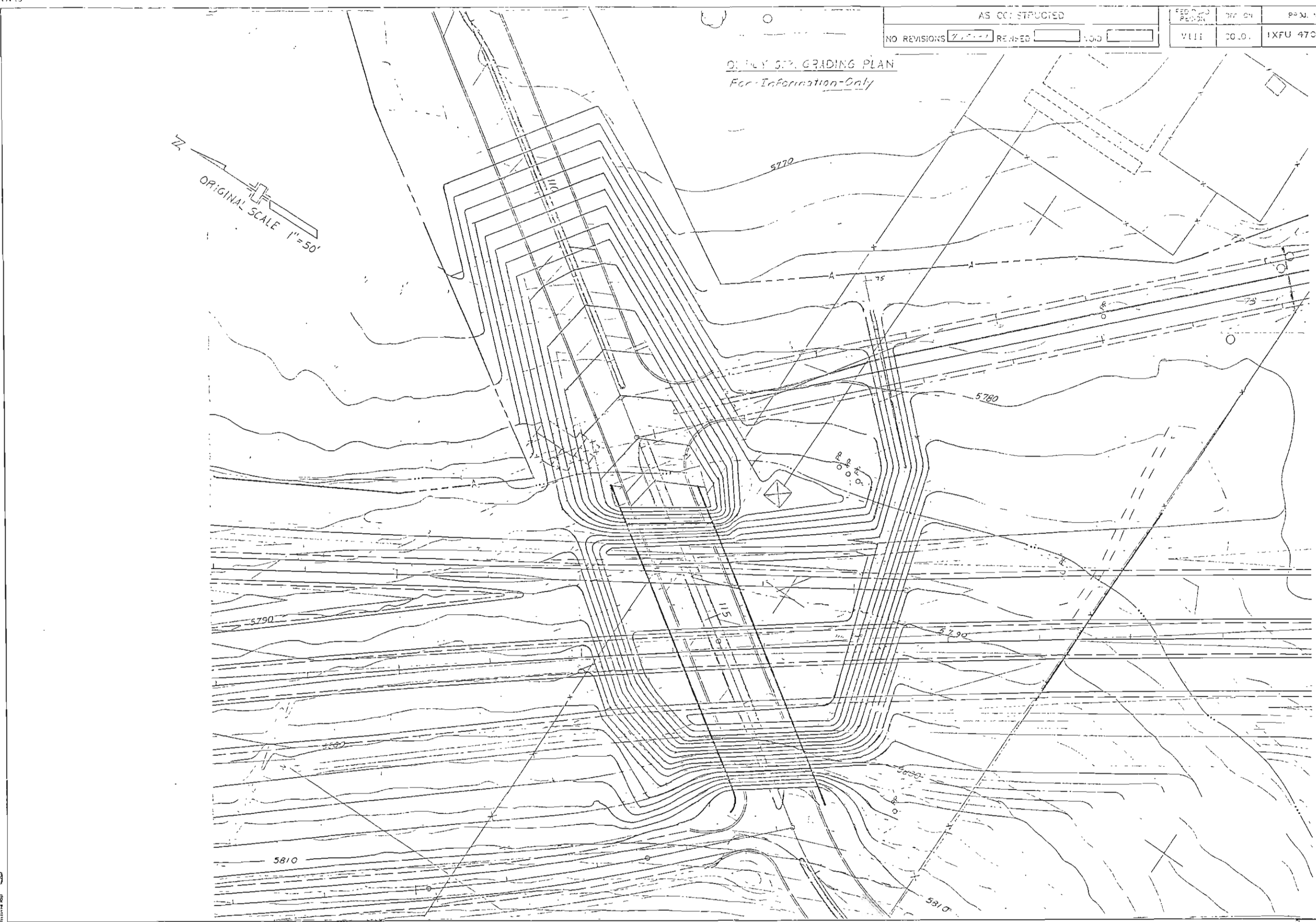
17 OF 200 2.0
15000 100 40
G. H. FOR 20.1
17 FEB 1984

AS CONSTRUCTED	
NO REVISIONS	REVISED
7/1/84	NO

FED. PROJ. NO.	DTM. NO.	PROJ. NO.	SHEET NO.	SHEET TOTALS
VIII	20.0	IXFU 470-1 (55)	18	41

ONLY SIZ. GRADING PLAN
For Information-Only

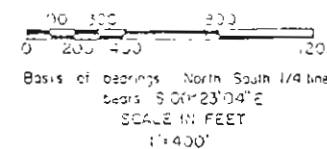
ORIGINAL SCALE 1"=50'



REGIONAL NO.	PROJECT NO.	DATE	BY
10	10-1001	40	41

RIGHT OF WAY CENTENNIAL PARKWAY (C-470) SH 255 to SH 85 Jefferson County

REVISIONS	DATE	BY
1.0/37.1 New sheet		DB
2.0/37.1 SH 255 to SH 85		DB
3.0/37.1 SH 255 to SH 85		DB
4.0/37.1 SH 255 to SH 85		DB
5.0/37.1 SH 255 to SH 85		DB
6.0/37.1 SH 255 to SH 85		DB
7.0/37.1 SH 255 to SH 85		DB
8.0/37.1 SH 255 to SH 85		DB
9.0/37.1 SH 255 to SH 85		DB
10.0/37.1 SH 255 to SH 85		DB



- 311 L. E. - R. L. H.
- 312 313 314 315 316 317 318 319 320 321 322 323 324 325 326 327 328 329 330 331 332 333 334 335 336 337 338 339 340 341 342 343 344 345 346 347 348 349 350 351 352 353 354 355 356 357 358 359 360 361 362 363 364 365 366 367 368 369 370 371 372 373 374 375 376 377 378 379 380 381 382 383 384 385 386 387 388 389 390 391 392 393 394 395 396 397 398 399 400 401 402 403 404 405 406 407 408 409 410 411 412 413 414 415 416 417 418 419 420 421 422 423 424 425 426 427 428 429 430 431 432 433 434 435 436 437 438 439 440 441 442 443 444 445 446 447 448 449 450 451 452 453 454 455 456 457 458 459 460 461 462 463 464 465 466 467 468 469 470 471 472 473 474 475 476 477 478 479 480 481 482 483 484 485 486 487 488 489 490 491 492 493 494 495 496 497 498 499 500 501 502 503 504 505 506 507 508 509 510 511 512 513 514 515 516 517 518 519 520 521 522 523 524 525 526 527 528 529 530 531 532 533 534 535 536 537 538 539 540 541 542 543 544 545 546 547 548 549 550 551 552 553 554 555 556 557 558 559 560 561 562 563 564 565 566 567 568 569 570 571 572 573 574 575 576 577 578 579 580 581 582 583 584 585 586 587 588 589 590 591 592 593 594 595 596 597 598 599 600 601 602 603 604 605 606 607 608 609 610 611 612 613 614 615 616 617 618 619 620 621 622 623 624 625 626 627 628 629 630 631 632 633 634 635 636 637 638 639 640 641 642 643 644 645 646 647 648 649 650 651 652 653 654 655 656 657 658 659 660 661 662 663 664 665 666 667 668 669 670 671 672 673 674 675 676 677 678 679 680 681 682 683 684 685 686 687 688 689 690 691 692 693 694 695 696 697 698 699 700 701 702 703 704 705 706 707 708 709 710 711 712 713 714 715 716 717 718 719 720 721 722 723 724 725 726 727 728 729 730 731 732 733 734 735 736 737 738 739 740 741 742 743 744 745 746 747 748 749 750 751 752 753 754 755 756 757 758 759 760 761 762 763 764 765 766 767 768 769 770 771 772 773 774 775 776 777 778 779 780 781 782 783 784 785 786 787 788 789 790 791 792 793 794 795 796 797 798 799 800 801 802 803 804 805 806 807 808 809 810 811 812 813 814 815 816 817 818 819 820 821 822 823 824 825 826 827 828 829 830 831 832 833 834 835 836 837 838 839 840 841 842 843 844 845 846 847 848 849 850 851 852 853 854 855 856 857 858 859 860 861 862 863 864 865 866 867 868 869 870 871 872 873 874 875 876 877 878 879 880 881 882 883 884 885 886 887 888 889 890 891 892 893 894 895 896 897 898 899 900 901 902 903 904 905 906 907 908 909 910 911 912 913 914 915 916 917 918 919 920 921 922 923 924 925 926 927 928 929 930 931 932 933 934 935 936 937 938 939 940 941 942 943 944 945 946 947 948 949 950 951 952 953 954 955 956 957 958 959 960 961 962 963 964 965 966 967 968 969 970 971 972 973 974 975 976 977 978 979 980 981 982 983 984 985 986 987 988 989 990 991 992 993 994 995 996 997 998 999 1000

OWNERSHIP MAP
PART OF
Sec 7, T.5S, R. 6W
Sec. 12, T.5S, R. 7W
JEFFERSON COUNTY
For Project IXFU 470-158